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- LEGEND
- SITE BOUNDARY
 - PROPOSED FOUL SEWER & MANHOLE
 - PROPOSED SURFACE WATER SEWER & MANHOLE
 - VORTEX FLOW CONTROL
 - PROPOSED PERMEABLE PAVING
 - PROPOSED HEADWALL
 - INDICATIVE ATTENUATION BASIN
 - BASIN EARTHWORKS EXTENT
 - INDICATIVE SWALE

FOR PLANNING PURPOSES ONLY

Rev

NA

15.12.20

Author

NA

15.12.20

Drawn

NA

15.12.20

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Job Title
HORTON HEATH, EASTLEIGH

Drawing Title
DRAINAGE STRATEGY

Client
EASTLEIGH BOROUGH COUNCIL

Scale
1:500 @A0

Date
NOV 20

Designed
NA

Drawn
JA

Checked
JW

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JH

Job No
19-367

Drawing No
19-367/001

Rev
A

P:\19-367 - Horton Heath, Eastleigh\Tech\Acad\Drawings\000 Series\19-367-001 Preliminary Engineering Layout.dwg

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Tuscany House White Hart Lane Basingstoke RG21 4AF	ONE HORTON HEATH BASIN J	
Date 15/12/2020 12:16 File 2020-11-02 Basin 7A and...	Designed by NA Checked by GG	
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STORM SEWER DESIGN by the Modified Rational Method

Design Criteria for Storm

Pipe Sizes STANDARD Manhole Sizes STANDARD

FSR Rainfall Model - England and Wales

Return Period (years)	100	PIMP (%)	100
M5-60 (mm)	19.900	Add Flow / Climate Change (%)	0
Ratio R	0.350	Minimum Backdrop Height (m)	0.200
Maximum Rainfall (mm/hr)	50	Maximum Backdrop Height (m)	1.500
Maximum Time of Concentration (mins)	30	Min Design Depth for Optimisation (m)	1.200
Foul Sewage (l/s/ha)	0.000	Min Vel for Auto Design only (m/s)	1.00
Volumetric Runoff Coeff.	0.750	Min Slope for Optimisation (1:X)	500

Designed with Level Soffits

Time Area Diagram for Storm

Time (mins)	Area (ha)	Time (mins)	Area (ha)
0-4	1.414	4-8	1.040

Total Area Contributing (ha) = 2.454

Total Pipe Volume (m³) = 45.831

Network Design Table for Storm

« - Indicates pipe capacity < flow

PN	Length (m)	Fall (m)	Slope (1:X)	I.Area (ha)	T.E. (mins)	Base Flow (l/s)	k (mm)	HYD SECT	DIA (mm)	Section Type	Auto Design
S1.000	40.587	0.338	120.0	0.119	4.00	0.0	0.600	o	300	Pipe/Conduit	
S1.001	25.643	0.154	167.0	0.168	0.00	0.0	0.600	o	300	Pipe/Conduit	
S1.002	8.304	0.073	113.8	0.055	0.00	0.0	0.600	o	300	Pipe/Conduit	
S1.003	31.868	0.878	36.3	0.000	0.00	0.0	0.600	o	300	Pipe/Conduit	

Network Results Table

PN	Rain (mm/hr)	T.C. (mins)	US/IL (m)	Σ I.Area (ha)	Σ Base Flow (l/s)	Foul (l/s)	Add Flow (l/s)	Vel (m/s)	Cap (l/s)	Flow (l/s)
S1.000	50.00	4.47	24.500	0.119	0.0	0.0	0.0	1.43	101.4	16.1
S1.001	50.00	4.82	24.162	0.287	0.0	0.0	0.0	1.21	85.8	38.9
S1.002	50.00	4.92	24.008	0.342	0.0	0.0	0.0	1.47	104.1	46.3
S1.003	50.00	5.12	23.935	0.342	0.0	0.0	0.0	2.62	185.0	46.3

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Network Design Table for Storm

PN	Length (m)	Fall (m)	Slope (1:X)	I.Area (ha)	T.E. (mins)	Base Flow (l/s)	k (mm)	HYD SECT	DIA (mm)	Section Type	Auto Design
S5.000	20.356	0.136	150.0	0.016	4.00	0.0	0.600	o	150	Pipe/Conduit	
S5.001	16.440	0.110	150.0	0.011	0.00	0.0	0.600	o	150	Pipe/Conduit	
S5.002	15.744	0.183	86.0	0.000	0.00	0.0	0.600	o	150	Pipe/Conduit	
S5.003	48.654	1.478	32.9	0.035	0.00	0.0	0.600	o	150	Pipe/Conduit	
S5.004	27.487	0.325	84.6	0.072	0.00	0.0	0.600	o	225	Pipe/Conduit	
S5.005	54.568	0.775	70.4	0.265	0.00	0.0	0.600	o	225	Pipe/Conduit	
S1.012	70.733	0.700	101.0	1.000	0.00	0.0	0.600	o	300	Pipe/Conduit	

Network Results Table

PN	Rain (mm/hr)	T.C. (mins)	US/IL (m)	Σ I.Area (ha)	Σ Base Flow (l/s)	Foul (l/s)	Add Flow (l/s)	Vel (m/s)	Cap (l/s)	Flow (l/s)
S5.000	50.00	4.41	23.906	0.016	0.0	0.0	0.0	0.82	14.5	2.2
S5.001	50.00	4.75	23.770	0.027	0.0	0.0	0.0	0.82	14.5	3.7
S5.002	50.00	4.99	23.661	0.027	0.0	0.0	0.0	1.08	19.2	3.7
S5.003	50.00	5.45	23.478	0.062	0.0	0.0	0.0	1.76	31.1	8.4
S5.004	50.00	5.77	21.925	0.134	0.0	0.0	0.0	1.42	56.6	18.1
S5.005	50.00	6.36	21.600	0.399	0.0	0.0	0.0	1.56	62.0	54.0
S1.012	50.00	7.65	20.600	2.454	0.0	0.0	0.0	1.56	110.6<	332.3

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Manhole Schedules for Storm

MH Name	MH CL (m)	MH Depth (m)	MH Connection	MH Diam., L*W (mm)	PN	Pipe Out Invert Level (m)	Pipe Out Diameter (mm)	PN	Pipes In Invert Level (m)	Pipes In Diameter (mm)	Backdrop (mm)
S1	26.000	1.500	Open Manhole	1200	S1.000	24.500	300				
S2	25.960	1.798	Open Manhole	1200	S1.001	24.162	300	S1.000	24.162	300	
S3	25.660	1.652	Open Manhole	1200	S1.002	24.008	300	S1.001	24.008	300	
S4	25.530	1.595	Open Manhole	1200	S1.003	23.935	300	S1.002	23.935	300	
S5	26.570	1.350	Open Manhole	1200	S2.000	25.220	150				
S6	25.500	1.425	Open Manhole	1200	S2.001	24.075	225	S2.000	24.150	150	
S7	25.120	1.425	Open Manhole	1200	S2.002	23.695	225	S2.001	23.695	225	
S8	24.950	1.425	Open Manhole	1200	S2.003	23.525	225	S2.002	23.525	225	
S5	24.720	1.737	Open Manhole	1350	S1.004	22.983	375	S1.003	23.058	300	
								S2.003	23.133	225	
S6	24.400	1.737	Open Manhole	1350	S1.005	22.663	375	S1.004	22.663	375	
S7	24.180	1.737	Open Manhole	1350	S1.006	22.443	375	S1.005	22.443	375	
S12	24.200	0.800	Open Manhole	1200	S3.000	23.400	225				
S13	24.100	0.760	Open Manhole	1200	S3.001	23.340	225	S3.000	23.340	225	
S8	23.670	1.737	Open Manhole	1350	S1.007	21.933	375	S1.006	21.933	375	
								S3.001	22.083	225	
S9	22.950	1.737	Open Manhole	1350	S1.008	21.213	375	S1.007	21.213	375	
S16	22.800	0.800	Open Manhole	1200	S4.000	22.000	225				
S17	22.770	0.794	Open Manhole	1200	S4.001	21.976	225	S4.000	21.976	225	
S10	22.740	1.737	Open Manhole	1350	S1.009	21.003	375	S1.008	21.003	375	
								S4.001	21.153	225	
S11	22.600	1.812	Open Manhole	1350	S1.010	20.788	450	S1.009	20.863	375	
S12	22.610	1.909	Open Manhole	1350	S1.011	20.701	450	S1.010	20.701	450	
S21	25.256	1.350	Open Manhole	1200	S5.000	23.906	150				
S22	25.560	1.789	Open Manhole	1200	S5.001	23.770	150	S5.000	23.770	150	
S23	25.629	1.969	Open Manhole	1200	S5.002	23.661	150	S5.001	23.661	150	
S24	24.828	1.350	Open Manhole	1200	S5.003	23.478	150	S5.002	23.478	150	
S25	23.350	1.425	Open Manhole	1200	S5.004	21.925	225	S5.003	22.000	150	
S26	22.500	0.900	Open Manhole	1200	S5.005	21.600	225	S5.004	21.600	225	
S13	21.600	1.000	Open Manhole	1350	S1.012	20.600	300	S1.011	20.600	450	
								S5.005	20.825	225	
S	20.300	0.400	Open Manhole	0		OUTFALL		S1.012	19.900	300	150

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Manhole Schedules for Storm

MH Name	Manhole Easting (m)	Manhole Northing (m)	Intersection Easting (m)	Intersection Northing (m)	Manhole Access	Layout (North)
S1	448968.087	116188.874	448968.087	116188.874	Required	
S2	448988.565	116223.916	448988.565	116223.916	Required	
S3	449000.622	116246.547	449000.622	116246.547	Required	
S4	448993.375	116250.601	448993.375	116250.601	Required	
S5	449078.021	116230.968	449078.021	116230.968	Required	
S6	449067.137	116251.551	449067.137	116251.551	Required	
S7	449047.734	116261.836	449047.734	116261.836	Required	
S8	449031.330	116265.933	449031.330	116265.933	Required	
S5	449009.624	116278.015	449009.624	116278.015	Required	
S6	449017.409	116292.046	449017.409	116292.046	Required	
S7	449023.687	116308.215	449023.687	116308.215	Required	
S12	449076.056	116305.735	449076.056	116305.735	Required	
S13	449067.426	116310.664	449067.426	116310.664	Required	
S8	449036.172	116328.513	449036.172	116328.513	Required	
S9	449054.673	116360.598	449054.673	116360.598	Required	

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Manhole Schedules for Storm

MH Name	Manhole Easting (m)	Manhole Northing (m)	Intersection Easting (m)	Intersection Northing (m)	Manhole Access	Layout (North)
S16	449066.690	116370.795	449066.690	116370.795	Required	
S17	449063.134	116370.755	449063.134	116370.755	Required	
S10	449056.066	116370.819	449056.066	116370.819	Required	
S11	449056.098	116384.723	449056.098	116384.723	Required	
S12	449066.588	116409.712	449066.588	116409.712	Required	
S21	449131.816	116327.692	449131.816	116327.692	Required	
S22	449141.616	116345.534	449141.616	116345.534	Required	
S23	449149.058	116360.193	449149.058	116360.193	Required	
S24	449144.224	116375.177	449144.224	116375.177	Required	
S25	449099.208	116393.637	449099.208	116393.637	Required	
S26	449109.638	116419.069	449109.638	116419.069	Required	
S13	449061.635	116445.019	449061.635	116445.019	Required	
S	448995.104	116469.033			No Entry	

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PIPELINE SCHEDULES for Storm

Upstream Manhole

PN	Hyd Sect	Diam (mm)	MH Name	C.Level (m)	I.Level (m)	D.Depth (m)	MH Connection	MH DIAM., L*W (mm)
S1.000	o	300	S1	26.000	24.500	1.200	Open Manhole	1200
S1.001	o	300	S2	25.960	24.162	1.498	Open Manhole	1200
S1.002	o	300	S3	25.660	24.008	1.352	Open Manhole	1200
S1.003	o	300	S4	25.530	23.935	1.295	Open Manhole	1200
S2.000	o	150	S5	26.570	25.220	1.200	Open Manhole	1200
S2.001	o	225	S6	25.500	24.075	1.200	Open Manhole	1200
S2.002	o	225	S7	25.120	23.695	1.200	Open Manhole	1200
S2.003	o	225	S8	24.950	23.525	1.200	Open Manhole	1200
S1.004	o	375	S5	24.720	22.983	1.362	Open Manhole	1350
S1.005	o	375	S6	24.400	22.663	1.362	Open Manhole	1350
S1.006	o	375	S7	24.180	22.443	1.362	Open Manhole	1350
S3.000	o	225	S12	24.200	23.400	0.575	Open Manhole	1200
S3.001	o	225	S13	24.100	23.340	0.535	Open Manhole	1200
S1.007	o	375	S8	23.670	21.933	1.362	Open Manhole	1350
S1.008	o	375	S9	22.950	21.213	1.362	Open Manhole	1350
S4.000	o	225	S16	22.800	22.000	0.575	Open Manhole	1200

Downstream Manhole

PN	Length (m)	Slope (1:X)	MH Name	C.Level (m)	I.Level (m)	D.Depth (m)	MH Connection	MH DIAM., L*W (mm)
S1.000	40.587	120.0	S2	25.960	24.162	1.498	Open Manhole	1200
S1.001	25.643	167.0	S3	25.660	24.008	1.352	Open Manhole	1200
S1.002	8.304	113.8	S4	25.530	23.935	1.295	Open Manhole	1200
S1.003	31.868	36.3	S5	24.720	23.058	1.362	Open Manhole	1350
S2.000	23.284	21.8	S6	25.500	24.150	1.200	Open Manhole	1200
S2.001	21.960	57.8	S7	25.120	23.695	1.200	Open Manhole	1200
S2.002	16.908	99.5	S8	24.950	23.525	1.200	Open Manhole	1200
S2.003	24.842	63.3	S5	24.720	23.133	1.362	Open Manhole	1350
S1.004	16.046	50.1	S6	24.400	22.663	1.362	Open Manhole	1350
S1.005	17.345	78.8	S7	24.180	22.443	1.362	Open Manhole	1350
S1.006	23.830	46.7	S8	23.670	21.933	1.362	Open Manhole	1350
S3.000	9.939	167.0	S13	24.100	23.340	0.535	Open Manhole	1200
S3.001	35.991	28.6	S8	23.670	22.083	1.362	Open Manhole	1350
S1.007	37.037	51.4	S9	22.950	21.213	1.362	Open Manhole	1350
S1.008	10.316	49.1	S10	22.740	21.003	1.362	Open Manhole	1350
S4.000	3.556	150.0	S17	22.770	21.976	0.569	Open Manhole	1200

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PIPELINE SCHEDULES for Storm

Upstream Manhole

PN	Hyd Sect	Diam (mm)	MH Name	C.Level (m)	I.Level (m)	D.Depth (m)	MH Connection	MH DIAM., L*W (mm)
S4.001	o	225	S17	22.770	21.976	0.569	Open Manhole	1200
S1.009	o	375	S10	22.740	21.003	1.362	Open Manhole	1350
S1.010	o	450	S11	22.600	20.788	1.362	Open Manhole	1350
S1.011	o	450	S12	22.610	20.701	1.459	Open Manhole	1350
S5.000	o	150	S21	25.256	23.906	1.200	Open Manhole	1200
S5.001	o	150	S22	25.560	23.770	1.639	Open Manhole	1200
S5.002	o	150	S23	25.629	23.661	1.819	Open Manhole	1200
S5.003	o	150	S24	24.828	23.478	1.200	Open Manhole	1200
S5.004	o	225	S25	23.350	21.925	1.200	Open Manhole	1200
S5.005	o	225	S26	22.500	21.600	0.675	Open Manhole	1200
S1.012	o	300	S13	21.600	20.600	0.700	Open Manhole	1350

Downstream Manhole

PN	Length (m)	Slope (1:X)	MH Name	C.Level (m)	I.Level (m)	D.Depth (m)	MH Connection	MH DIAM., L*W (mm)
S4.001	7.069	8.6	S10	22.740	21.153	1.362	Open Manhole	1350
S1.009	13.903	99.3	S11	22.600	20.863	1.362	Open Manhole	1350
S1.010	27.102	313.0	S12	22.610	20.701	1.459	Open Manhole	1350
S1.011	35.652	353.0	S13	21.600	20.600	0.550	Open Manhole	1350
S5.000	20.356	150.0	S22	25.560	23.770	1.639	Open Manhole	1200
S5.001	16.440	150.0	S23	25.629	23.661	1.819	Open Manhole	1200
S5.002	15.744	86.0	S24	24.828	23.478	1.200	Open Manhole	1200
S5.003	48.654	32.9	S25	23.350	22.000	1.200	Open Manhole	1200
S5.004	27.487	84.6	S26	22.500	21.600	0.675	Open Manhole	1200
S5.005	54.568	70.4	S13	21.600	20.825	0.550	Open Manhole	1350
S1.012	70.733	101.0	S	20.300	19.900	0.100	Open Manhole	0

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Area Summary for Storm

Pipe Number	PIMP Type	PIMP Name	PIMP (%)	Gross Area (ha)	Imp. Area (ha)	Pipe Total (ha)
1.000	-	-	100	0.119	0.119	0.119
1.001	-	-	100	0.168	0.168	0.168
1.002	-	-	100	0.055	0.055	0.055
1.003	-	-	100	0.000	0.000	0.000
2.000	-	-	100	0.026	0.026	0.026
2.001	-	-	100	0.022	0.022	0.022
2.002	-	-	100	0.057	0.057	0.057
2.003	-	-	100	0.037	0.037	0.037
1.004	-	-	100	0.055	0.055	0.055
1.005	-	-	100	0.000	0.000	0.000
1.006	-	-	100	0.047	0.047	0.047
3.000	-	-	100	0.117	0.117	0.117
3.001	-	-	100	0.000	0.000	0.000
1.007	-	-	100	0.097	0.097	0.097
1.008	-	-	100	0.062	0.062	0.062
4.000	-	-	100	0.155	0.155	0.155
4.001	-	-	100	0.000	0.000	0.000
1.009	-	-	100	0.000	0.000	0.000
1.010	-	-	100	0.012	0.012	0.012
1.011	-	-	100	0.026	0.026	0.026
5.000	-	-	100	0.016	0.016	0.016
5.001	-	-	100	0.011	0.011	0.011
5.002	-	-	100	0.000	0.000	0.000
5.003	-	-	100	0.035	0.035	0.035
5.004	-	-	100	0.072	0.072	0.072
5.005	-	-	100	0.265	0.265	0.265
1.012	-	-	100	1.000	1.000	1.000
				Total	Total	Total
				2.454	2.454	2.454

Free Flowing Outfall Details for Storm

Outfall Pipe Number	Outfall Name	C. Level (m)	I. Level (m)	Min I. Level (m)	D,L (mm)	W (mm)
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S1.012	S	20.300	19.900	0.000	0	0
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Simulation Criteria for Storm

Volumetric Runoff Coeff	0.750	Additional Flow - % of Total Flow	0.000
Areal Reduction Factor	1.000	MADD Factor * 10m³/ha Storage	2.000
Hot Start (mins)	0	Inlet Coefficient	0.800
Hot Start Level (mm)	0	Flow per Person per Day (l/per/day)	0.000
Manhole Headloss Coeff (Global)	0.500	Run Time (mins)	60
Foul Sewage per hectare (l/s)	0.000	Output Interval (mins)	1
Number of Input Hydrographs	0	Number of Offline Controls	0
Number of Online Controls	4	Number of Storage Structures	4

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Simulation Criteria for Storm

Number of Time/Area Diagrams 0 Number of Real Time Controls 0

Synthetic Rainfall Details

Rainfall Model	FSR	Profile Type	Summer
Return Period (years)	100	Cv (Summer)	0.750
Region	England and Wales	Cv (Winter)	0.840
M5-60 (mm)	19.900	Storm Duration (mins)	30
Ratio R	0.350		

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Online Controls for Storm

Hydro-Brake® Optimum Manhole: S13, DS/PN: S3.001, Volume (m³): 1.2

Unit Reference MD-SHE-0137-8000-0560-8000
Design Head (m) 0.560
Design Flow (l/s) 8.0
Flush-Flo™ Calculated
Objective Minimise upstream storage
Application Surface
Sump Available Yes
Diameter (mm) 137
Invert Level (m) 23.340
Minimum Outlet Pipe Diameter (mm) 150
Suggested Manhole Diameter (mm) 1200

Control Points	Head (m)	Flow (l/s)
Design Point (Calculated)	0.560	8.0
Flush-Flo™	0.216	8.0
Kick-Flo®	0.424	7.0
Mean Flow over Head Range	-	6.5

The hydrological calculations have been based on the Head/Discharge relationship for the Hydro-Brake® Optimum as specified. Should another type of control device other than a Hydro-Brake Optimum® be utilised then these storage routing calculations will be invalidated

Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)
0.100	5.0	1.200	11.4	3.000	17.7	7.000	26.6
0.200	8.0	1.400	12.3	3.500	19.1	7.500	27.5
0.300	7.8	1.600	13.1	4.000	20.3	8.000	28.4
0.400	7.3	1.800	13.9	4.500	21.5	8.500	29.3
0.500	7.6	2.000	14.6	5.000	22.6	9.000	30.2
0.600	8.3	2.200	15.3	5.500	23.7	9.500	31.0
0.800	9.5	2.400	15.9	6.000	24.6		
1.000	10.5	2.600	16.5	6.500	25.6		

Hydro-Brake® Optimum Manhole: S17, DS/PN: S4.001, Volume (m³): 1.0

Unit Reference MD-SHE-0209-2100-0524-2100
Design Head (m) 0.524
Design Flow (l/s) 21.0
Flush-Flo™ Calculated
Objective Minimise upstream storage
Application Surface
Sump Available Yes
Diameter (mm) 209
Invert Level (m) 21.976
Minimum Outlet Pipe Diameter (mm) 225
Suggested Manhole Diameter (mm) 1200

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Hydro-Brake® Optimum Manhole: S17, DS/PN: S4.001, Volume (m³): 1.0

Control Points	Head (m)	Flow (l/s)
Design Point (Calculated)	0.524	21.0
Flush-Flo™	0.293	21.0
Kick-Flo®	0.448	19.5
Mean Flow over Head Range	-	15.8

The hydrological calculations have been based on the Head/Discharge relationship for the Hydro-Brake® Optimum as specified. Should another type of control device other than a Hydro-Brake Optimum® be utilised then these storage routing calculations will be invalidated

Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)
0.100	7.1	1.200	31.2	3.000	48.5	7.000	73.0
0.200	20.1	1.400	33.6	3.500	52.3	7.500	75.6
0.300	21.0	1.600	35.8	4.000	55.8	8.000	78.1
0.400	20.3	1.800	37.9	4.500	59.1	8.500	80.6
0.500	20.5	2.000	39.9	5.000	62.2	9.000	82.9
0.600	22.4	2.200	41.8	5.500	64.6	9.500	85.2
0.800	25.7	2.400	43.6	6.000	67.5		
1.000	28.6	2.600	45.3	6.500	70.3		

Hydro-Brake® Optimum Manhole: S26, DS/PN: S5.005, Volume (m³): 2.1

Unit Reference	MD-SHE-0048-1000-0900-1000
Design Head (m)	0.900
Design Flow (l/s)	1.0
Flush-Flo™	Calculated
Objective	Minimise upstream storage
Application	Surface
Sump Available	Yes
Diameter (mm)	48
Invert Level (m)	21.600
Minimum Outlet Pipe Diameter (mm)	75
Suggested Manhole Diameter (mm)	1200

Control Points	Head (m)	Flow (l/s)
Design Point (Calculated)	0.900	1.0
Flush-Flo™	0.212	0.9
Kick-Flo®	0.428	0.7
Mean Flow over Head Range	-	0.8

The hydrological calculations have been based on the Head/Discharge relationship for the Hydro-Brake® Optimum as specified. Should another type of control device other than a Hydro-Brake Optimum® be utilised then these storage routing calculations will be invalidated

Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)
0.100	0.8	0.300	0.9	0.500	0.8	0.800	0.9
0.200	0.9	0.400	0.8	0.600	0.8	1.000	1.0

Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)
1.200	1.1	2.400	1.6	5.000	2.2	8.000	2.7
1.400	1.2	2.600	1.6	5.500	2.3	8.500	2.8
1.600	1.3	3.000	1.7	6.000	2.4	9.000	2.9
1.800	1.4	3.500	1.8	6.500	2.5	9.500	2.9
2.000	1.4	4.000	2.0	7.000	2.5		
2.200	1.5	4.500	2.1	7.500	2.6		

Unit Reference	MD-SHE-0235-2960-1000-2960
Design Head (m)	1.000
Design Flow (l/s)	29.6
Flush-Flo™	Calculated
Objective	Minimise upstream storage
Application	Surface
Sump Available	Yes
Diameter (mm)	235
Invert Level (m)	20.600
Minimum Outlet Pipe Diameter (mm)	300
Suggested Manhole Diameter (mm)	1500

The hydrological calculations have been based on the Head/Discharge relationship for the Hydro-Brake® Optimum as specified. Should another type of control device other than a Hydro-Brake Optimum® be utilised then these storage routing calculations will be invalidated

Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)
0.100	7.8	1.200	32.3	3.000	50.2	7.000	75.7
0.200	23.7	1.400	34.8	3.500	54.1	7.500	78.3
0.300	29.2	1.600	37.1	4.000	57.7	8.000	80.8
0.400	29.5	1.800	39.2	4.500	61.1	8.500	83.2
0.500	29.1	2.000	41.3	5.000	64.3	9.000	85.6
0.600	28.4	2.200	43.2	5.500	67.3	9.500	87.9
0.800	26.6	2.400	45.1	6.000	70.3		
1.000	29.6	2.600	46.8	6.500	73.0		

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Storage Structures for Storm

Porous Car Park Manhole: S12, DS/PN: S3.000

Infiltration Coefficient Base (m/hr)	0.00000	Width (m)	20.0
Membrane Percolation (mm/hr)	1000	Length (m)	40.1
Max Percolation (l/s)	222.8	Slope (1:X)	40.0
Safety Factor	2.0	Depression Storage (mm)	5
Porosity	0.30	Evaporation (mm/day)	3
Invert Level (m)	23.400	Membrane Depth (mm)	0

Porous Car Park Manhole: S16, DS/PN: S4.000

Infiltration Coefficient Base (m/hr)	0.00000	Width (m)	17.1
Membrane Percolation (mm/hr)	1000	Length (m)	80.0
Max Percolation (l/s)	380.0	Slope (1:X)	40.0
Safety Factor	2.0	Depression Storage (mm)	5
Porosity	0.30	Evaporation (mm/day)	3
Invert Level (m)	22.000	Membrane Depth (mm)	0

Tank or Pond Manhole: S26, DS/PN: S5.005

Invert Level (m) 21.600

Depth (m)	Area (m ²)	Depth (m)	Area (m ²)
0.000	280.0	0.900	592.0

Tank or Pond Manhole: S13, DS/PN: S1.012

Invert Level (m) 20.600

Depth (m)	Area (m ²)	Depth (m)	Area (m ²)
0.000	919.0	1.000	1624.0

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STORM SEWER DESIGN by the Modified Rational Method

Design Criteria for Storm

Pipe Sizes STANDARD Manhole Sizes STANDARD

FSR Rainfall Model - England and Wales

Return Period (years)	100	PIMP (%)	100
M5-60 (mm)	19.900	Add Flow / Climate Change (%)	0
Ratio R	0.350	Minimum Backdrop Height (m)	0.200
Maximum Rainfall (mm/hr)	50	Maximum Backdrop Height (m)	1.500
Maximum Time of Concentration (mins)	30	Min Design Depth for Optimisation (m)	1.200
Foul Sewage (l/s/ha)	0.000	Min Vel for Auto Design only (m/s)	1.00
Volumetric Runoff Coeff.	0.750	Min Slope for Optimisation (1:X)	500

Designed with Level Soffits

Time Area Diagram for Storm

Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)
0-4	0.405	4-8	1.058	8-12	0.062

Total Area Contributing (ha) = 1.526

Total Pipe Volume (m³) = 77.938

Network Design Table for Storm

« - Indicates pipe capacity < flow

PN	Length (m)	Fall (m)	Slope (1:X)	I.Area (ha)	T.E. (mins)	Base Flow (l/s)	k (mm)	HYD SECT	DIA (mm)	Section Type	Auto Design
S1.000	22.797	0.250	91.2	0.023	4.00	0.0	0.600	o	150	Pipe/Conduit	
S1.001	19.510	0.500	39.0	0.020	0.00	0.0	0.600	o	150	Pipe/Conduit	
S2.000	11.397	0.076	150.0	0.044	4.00	0.0	0.600	o	150	Pipe/Conduit	
S2.001	30.322	0.202	150.0	0.000	0.00	0.0	0.600	o	150	Pipe/Conduit	

Network Results Table

PN	Rain (mm/hr)	T.C. (mins)	US/IL (m)	Σ I.Area (ha)	Σ Base Flow (l/s)	Foul (l/s)	Add Flow (l/s)	Vel (m/s)	Cap (l/s)	Flow (l/s)
S1.000	50.00	4.36	23.300	0.023	0.0	0.0	0.0	1.05	18.6	3.1
S1.001	50.00	4.56	23.050	0.043	0.0	0.0	0.0	1.62	28.6	5.8
S2.000	50.00	4.23	23.630	0.044	0.0	0.0	0.0	0.82	14.5	6.0
S2.001	50.00	4.85	23.554	0.044	0.0	0.0	0.0	0.82	14.5	6.0

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Network Design Table for Storm

PN	Length (m)	Fall (m)	Slope (1:X)	I.Area (ha)	T.E. (mins)	Base Flow (l/s)	k (mm)	HYD SECT	DIA (mm)	Section Type	Auto Design
S1.008	49.622	0.124	400.2	0.132	0.00	0.0	0.600	o	525	Pipe/Conduit	
S1.009	58.552	0.101	579.7	0.143	0.00	0.0	0.600	o	525	Pipe/Conduit	
S1.010	24.688	0.700	35.3	0.000	0.00	0.0	0.600	o	375	Pipe/Conduit	

Network Results Table

PN	Rain (mm/hr)	T.C. (mins)	US/IL (m)	Σ I.Area (ha)	Σ Base Flow (l/s)	Foul (l/s)	Add Flow (l/s)	Vel (m/s)	Cap (l/s)	Flow (l/s)
S1.008	50.00	8.82	19.125	1.383	0.0	0.0	0.0	1.11	241.0	187.3
S1.009	50.00	9.88	19.001	1.526	0.0	0.0	0.0	0.92	199.8	206.6
S1.010	50.00	10.01	18.900	1.526	0.0	0.0	0.0	3.06	337.9	206.6

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Manhole Schedules for Storm

MH Name	MH CL (m)	MH Depth (m)	MH Connection	MH Diam., L*W (mm)	PN	Pipe Out Invert Level (m)	Pipe Out Diameter (mm)	PN	Pipes In Invert Level (m)	Pipes In Diameter (mm)	Backdrop (mm)
S1	24.650	1.350	Open Manhole	1200	S1.000	23.300	150				
S2	24.400	1.350	Open Manhole	1200	S1.001	23.050	150	S1.000	23.050	150	
S3	24.680	1.050	Open Manhole	1200	S2.000	23.630	150				
S4	24.550	0.996	Open Manhole	1200	S2.001	23.554	150	S2.000	23.554	150	
S5	24.230	0.878	Open Manhole	1200	S2.002	23.352	150	S2.001	23.352	150	
S6	23.950	0.800	Open Manhole	1200	S2.003	23.150	150	S2.002	23.150	150	
S3	23.900	1.425	Open Manhole	1200	S1.002	22.475	225	S1.001	22.550	150	
								S2.003	22.550	150	
S4	23.100	1.425	Open Manhole	1200	S1.003	21.675	225	S1.002	21.675	225	
S5	22.750	1.500	Open Manhole	1200	S1.004	21.250	300	S1.003	21.325	225	
S6	22.600	1.607	Open Manhole	1200	S1.005	20.993	300	S1.004	20.993	300	
S11	22.960	1.500	Open Manhole	1200	S3.000	21.460	300				
S12	22.600	1.500	Open Manhole	1200	S3.001	21.100	300	S3.000	21.100	300	
S13	21.890	1.575	Open Manhole	1350	S3.002	20.315	375	S3.001	20.390	300	
S7	21.550	1.850	Open Manhole	1350	S1.006	19.700	450	S1.005	19.850	300	
								S3.002	19.775	375	
S8	21.300	1.774	Open Manhole	1350	S1.007	19.526	450	S1.006	19.526	450	
S16	22.890	1.425	Open Manhole	1200	S4.000	21.465	225				
S17	22.580	1.575	Open Manhole	1350	S4.001	21.005	375	S4.000	21.155	225	
S18	22.100	1.575	Open Manhole	1350	S4.002	20.525	375	S4.001	20.525	375	
S19	21.850	1.575	Open Manhole	1350	S4.003	20.275	375	S4.002	20.275	375	
S20	21.680	1.575	Open Manhole	1350	S4.004	20.105	375	S4.003	20.105	375	
S21	21.280	1.650	Open Manhole	1350	S4.005	19.630	450	S4.004	19.705	375	
S9	20.950	1.825	Open Manhole	1500	S1.008	19.125	525	S1.007	19.200	450	
								S4.005	19.200	450	
S10	21.150	2.149	Open Manhole	1500	S1.009	19.001	525	S1.008	19.001	525	
S11	20.100	1.200	Open Manhole	1500	S1.010	18.900	375	S1.009	18.900	525	
S	18.800	0.600	Open Manhole	0		OUTFALL		S1.010	18.200	375	

MH Name	Manhole Easting (m)	Manhole Northing (m)	Intersection Easting (m)	Intersection Northing (m)	Manhole Access	Layout (North)
S1	449008.038	116287.947	449008.038	116287.947	Required	
S2	449019.154	116307.850	449019.154	116307.850	Required	

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Manhole Schedules for Storm

MH Name	Manhole Easting (m)	Manhole Northing (m)	Intersection Easting (m)	Intersection Northing (m)	Manhole Access	Layout (North)
S18	448889.321	116385.498	448889.321	116385.498	Required	
S19	448881.742	116398.436	448881.742	116398.436	Required	
S20	448882.734	116406.779	448882.734	116406.779	Required	
S21	448896.297	116437.081	448896.297	116437.081	Required	
S9	448907.926	116467.214	448907.926	116467.214	Required	
S10	448862.345	116486.826	448862.345	116486.826	Required	
S11	448827.261	116533.703	448827.261	116533.703	Required	
S	448828.439	116558.363			No Entry	

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PIPELINE SCHEDULES for Storm

Upstream Manhole

PN	Hyd Sect	Diam (mm)	MH Name	C.Level (m)	I.Level (m)	D.Depth (m)	MH Connection	MH DIAM., L*W (mm)
S1.000	o	150	S1	24.650	23.300	1.200	Open Manhole	1200
S1.001	o	150	S2	24.400	23.050	1.200	Open Manhole	1200
S2.000	o	150	S3	24.680	23.630	0.900	Open Manhole	1200
S2.001	o	150	S4	24.550	23.554	0.846	Open Manhole	1200
S2.002	o	150	S5	24.230	23.352	0.728	Open Manhole	1200
S2.003	o	150	S6	23.950	23.150	0.650	Open Manhole	1200
S1.002	o	225	S3	23.900	22.475	1.200	Open Manhole	1200
S1.003	o	225	S4	23.100	21.675	1.200	Open Manhole	1200
S1.004	o	300	S5	22.750	21.250	1.200	Open Manhole	1200
S1.005	o	300	S6	22.600	20.993	1.307	Open Manhole	1200
S3.000	o	300	S11	22.960	21.460	1.200	Open Manhole	1200
S3.001	o	300	S12	22.600	21.100	1.200	Open Manhole	1200
S3.002	o	375	S13	21.890	20.315	1.200	Open Manhole	1350
S1.006	o	450	S7	21.550	19.700	1.400	Open Manhole	1350
S1.007	o	450	S8	21.300	19.526	1.324	Open Manhole	1350
S4.000	o	225	S16	22.890	21.465	1.200	Open Manhole	1200

Downstream Manhole

PN	Length (m)	Slope (1:X)	MH Name	C.Level (m)	I.Level (m)	D.Depth (m)	MH Connection	MH DIAM., L*W (mm)
S1.000	22.797	91.2	S2	24.400	23.050	1.200	Open Manhole	1200
S1.001	19.510	39.0	S3	23.900	22.550	1.200	Open Manhole	1200
S2.000	11.397	150.0	S4	24.550	23.554	0.846	Open Manhole	1200
S2.001	30.322	150.0	S5	24.230	23.352	0.728	Open Manhole	1200
S2.002	26.658	132.0	S6	23.950	23.150	0.650	Open Manhole	1200
S2.003	6.973	11.6	S3	23.900	22.550	1.200	Open Manhole	1200
S1.002	31.372	39.2	S4	23.100	21.675	1.200	Open Manhole	1200
S1.003	18.438	52.7	S5	22.750	21.325	1.200	Open Manhole	1200
S1.004	42.815	167.0	S6	22.600	20.993	1.307	Open Manhole	1200
S1.005	76.810	67.2	S7	21.550	19.850	1.400	Open Manhole	1350
S3.000	24.924	69.2	S12	22.600	21.100	1.200	Open Manhole	1200
S3.001	30.693	43.2	S13	21.890	20.390	1.200	Open Manhole	1350
S3.002	33.968	62.9	S7	21.550	19.775	1.400	Open Manhole	1350
S1.006	43.520	250.1	S8	21.300	19.526	1.324	Open Manhole	1350
S1.007	45.500	139.6	S9	20.950	19.200	1.300	Open Manhole	1500
S4.000	28.917	93.3	S17	22.580	21.155	1.200	Open Manhole	1350

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PIPELINE SCHEDULES for Storm

Upstream Manhole

PN	Hyd Sect	Diam (mm)	MH Name	C.Level (m)	I.Level (m)	D.Depth (m)	MH Connection	MH DIAM., L*W (mm)
S4.001	o	375	S17	22.580	21.005	1.200	Open Manhole	1350
S4.002	o	375	S18	22.100	20.525	1.200	Open Manhole	1350
S4.003	o	375	S19	21.850	20.275	1.200	Open Manhole	1350
S4.004	o	375	S20	21.680	20.105	1.200	Open Manhole	1350
S4.005	o	450	S21	21.280	19.630	1.200	Open Manhole	1350
S1.008	o	525	S9	20.950	19.125	1.300	Open Manhole	1500
S1.009	o	525	S10	21.150	19.001	1.624	Open Manhole	1500
S1.010	o	375	S11	20.100	18.900	0.825	Open Manhole	1500

Downstream Manhole

PN	Length (m)	Slope (1:X)	MH Name	C.Level (m)	I.Level (m)	D.Depth (m)	MH Connection	MH DIAM., L*W (mm)
S4.001	44.372	92.4	S18	22.100	20.525	1.200	Open Manhole	1350
S4.002	14.994	60.0	S19	21.850	20.275	1.200	Open Manhole	1350
S4.003	8.401	49.4	S20	21.680	20.105	1.200	Open Manhole	1350
S4.004	33.199	83.0	S21	21.280	19.705	1.200	Open Manhole	1350
S4.005	32.300	75.1	S9	20.950	19.200	1.300	Open Manhole	1500
S1.008	49.622	400.2	S10	21.150	19.001	1.624	Open Manhole	1500
S1.009	58.552	579.7	S11	20.100	18.900	0.675	Open Manhole	1500
S1.010	24.688	35.3	S	18.800	18.200	0.225	Open Manhole	0

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Area Summary for Storm

Pipe Number	PIMP Type	PIMP Name	PIMP (%)	Gross Area (ha)	Imp. Area (ha)	Pipe Total (ha)
1.000	-	-	100	0.023	0.023	0.023
1.001	-	-	100	0.020	0.020	0.020
2.000	-	-	100	0.044	0.044	0.044
2.001	-	-	100	0.000	0.000	0.000
2.002	-	-	100	0.000	0.000	0.000
2.003	-	-	100	0.132	0.132	0.132
1.002	-	-	100	0.019	0.019	0.019
1.003	-	-	100	0.039	0.039	0.039
1.004	-	-	100	0.023	0.023	0.023
1.005	-	-	100	0.082	0.082	0.082
3.000	-	-	100	0.113	0.113	0.113
3.001	-	-	100	0.044	0.044	0.044
3.002	-	-	100	0.085	0.085	0.085
1.006	-	-	100	0.137	0.137	0.137
1.007	-	-	100	0.000	0.000	0.000
4.000	-	-	100	0.063	0.063	0.063
4.001	-	-	100	0.158	0.158	0.158
4.002	-	-	100	0.073	0.073	0.073
4.003	-	-	100	0.019	0.019	0.019
4.004	-	-	100	0.083	0.083	0.083
4.005	-	-	100	0.094	0.094	0.094
1.008	-	-	100	0.132	0.132	0.132
1.009	-	-	100	0.143	0.143	0.143
1.010	-	-	100	0.000	0.000	0.000
				Total	Total	Total
				1.526	1.526	1.526

Free Flowing Outfall Details for Storm

Outfall Pipe Number	Outfall Name	C. Level (m)	I. Level (m)	Min I. Level (m)	D,L (mm)	W (mm)
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S1.010	S	18.800	18.200	0.000	0	0
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Simulation Criteria for Storm

Volumetric Runoff Coeff	0.750	Additional Flow - % of Total Flow	0.000
Areal Reduction Factor	1.000	MADD Factor * 10m ³ /ha Storage	2.000
Hot Start (mins)	0	Inlet Coefficient	0.800
Hot Start Level (mm)	0	Flow per Person per Day (l/per/day)	0.000
Manhole Headloss Coeff (Global)	0.500	Run Time (mins)	60
Foul Sewage per hectare (l/s)	0.000	Output Interval (mins)	1
Number of Input Hydrographs	0	Number of Storage Structures	2
Number of Online Controls	2	Number of Time/Area Diagrams	0
Number of Offline Controls	0	Number of Real Time Controls	0

Synthetic Rainfall Details

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Simulation Criteria for Storm <table><tr><td>Rainfall Model</td><td>FSR</td><td>Profile Type</td><td>Summer</td></tr><tr><td>Return Period (years)</td><td>100</td><td>Cv (Summer)</td><td>0.750</td></tr><tr><td>Region</td><td>England and Wales</td><td>Cv (Winter)</td><td>0.840</td></tr><tr><td>M5-60 (mm)</td><td>19.900</td><td>Storm Duration (mins)</td><td>30</td></tr><tr><td>Ratio R</td><td>0.350</td><td></td><td></td></tr></table>			Rainfall Model	FSR	Profile Type	Summer	Return Period (years)	100	Cv (Summer)	0.750	Region	England and Wales	Cv (Winter)	0.840	M5-60 (mm)	19.900	Storm Duration (mins)	30	Ratio R	0.350		
Rainfall Model	FSR	Profile Type	Summer																			
Return Period (years)	100	Cv (Summer)	0.750																			
Region	England and Wales	Cv (Winter)	0.840																			
M5-60 (mm)	19.900	Storm Duration (mins)	30																			
Ratio R	0.350																					
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Online Controls for Storm

Hydro-Brake® Optimum Manhole: S6, DS/PN: S2.003, Volume (m³): 1.4

Unit Reference MD-SHE-0090-3000-0500-3000
Design Head (m) 0.500
Design Flow (l/s) 3.0
Flush-Flo™ Calculated
Objective Minimise upstream storage
Application Surface
Sump Available Yes
Diameter (mm) 90
Invert Level (m) 23.150
Minimum Outlet Pipe Diameter (mm) 150
Suggested Manhole Diameter (mm) 1200

Control Points	Head (m)	Flow (l/s)
Design Point (Calculated)	0.500	3.0
Flush-Flo™	0.157	3.0
Kick-Flo®	0.355	2.6
Mean Flow over Head Range	-	2.5

The hydrological calculations have been based on the Head/Discharge relationship for the Hydro-Brake® Optimum as specified. Should another type of control device other than a Hydro-Brake Optimum® be utilised then these storage routing calculations will be invalidated

Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)
0.100	2.8	1.200	4.5	3.000	6.9	7.000	10.4
0.200	3.0	1.400	4.8	3.500	7.4	7.500	10.7
0.300	2.8	1.600	5.1	4.000	7.9	8.000	11.1
0.400	2.7	1.800	5.4	4.500	8.4	8.500	11.4
0.500	3.0	2.000	5.7	5.000	8.8	9.000	11.8
0.600	3.3	2.200	6.0	5.500	9.2	9.500	12.1
0.800	3.7	2.400	6.2	6.000	9.6		
1.000	4.1	2.600	6.4	6.500	10.0		

Hydro-Brake® Optimum Manhole: S11, DS/PN: S1.010, Volume (m³): 14.5

Unit Reference MD-SHE-0165-1360-1200-1360
Design Head (m) 1.200
Design Flow (l/s) 13.6
Flush-Flo™ Calculated
Objective Minimise upstream storage
Application Surface
Sump Available Yes
Diameter (mm) 165
Invert Level (m) 18.900
Minimum Outlet Pipe Diameter (mm) 225
Suggested Manhole Diameter (mm) 1500

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Hydro-Brake® Optimum Manhole: S11, DS/PN: S1.010, Volume (m³): 14.5

Control Points	Head (m)	Flow (l/s)
Design Point (Calculated)	1.200	13.6
Flush-Flo™	0.364	13.6
Kick-Flo®	0.801	11.2
Mean Flow over Head Range	-	11.7

The hydrological calculations have been based on the Head/Discharge relationship for the Hydro-Brake® Optimum as specified. Should another type of control device other than a Hydro-Brake Optimum® be utilised then these storage routing calculations will be invalidated

Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)
0.100	5.9	1.200	13.6	3.000	21.0	7.000	31.6
0.200	12.8	1.400	14.6	3.500	22.6	7.500	32.7
0.300	13.5	1.600	15.6	4.000	24.1	8.000	33.7
0.400	13.6	1.800	16.5	4.500	25.5	8.500	34.7
0.500	13.4	2.000	17.3	5.000	26.9	9.000	35.7
0.600	13.1	2.200	18.1	5.500	28.1	9.500	36.6
0.800	11.3	2.400	18.9	6.000	29.3		
1.000	12.5	2.600	19.6	6.500	30.5		

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Storage Structures for Storm

Porous Car Park Manhole: S6, DS/PN: S2.003

Infiltration Coefficient Base (m/hr)	0.00000	Width (m)	92.3
Membrane Percolation (mm/hr)	1000	Length (m)	12.4
Max Percolation (l/s)	317.9	Slope (1:X)	100.0
Safety Factor	2.0	Depression Storage (mm)	5
Porosity	0.30	Evaporation (mm/day)	3
Invert Level (m)	23.150	Membrane Depth (mm)	0

Tank or Pond Manhole: S11, DS/PN: S1.010

Invert Level (m) 18.900

Depth (m)	Area (m ²)	Depth (m)	Area (m ²)
0.000	807.0	1.200	1502.0

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STORM SEWER DESIGN by the Modified Rational Method

Design Criteria for Storm

Pipe Sizes STANDARD Manhole Sizes STANDARD

FSR Rainfall Model - England and Wales

Return Period (years)	100	PIMP (%)	100
M5-60 (mm)	19.900	Add Flow / Climate Change (%)	0
Ratio R	0.350	Minimum Backdrop Height (m)	0.200
Maximum Rainfall (mm/hr)	50	Maximum Backdrop Height (m)	1.500
Maximum Time of Concentration (mins)	30	Min Design Depth for Optimisation (m)	1.200
Foul Sewage (l/s/ha)	0.000	Min Vel for Auto Design only (m/s)	1.00
Volumetric Runoff Coeff.	0.750	Min Slope for Optimisation (1:X)	500

Designed with Level Soffits

Time Area Diagram for Storm

Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)
0-4	0.035	4-8	0.771	8-12	1.500	12-16	0.026

Total Area Contributing (ha) = 2.332

Total Pipe Volume (m³) = 207.839

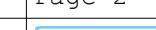
Network Design Table for Storm

« - Indicates pipe capacity < flow

PN	Length (m)	Fall (m)	Slope (1:X)	I.Area (ha)	T.E. (mins)	Base Flow (l/s)	k (mm)	HYD SECT	DIA (mm)	Section Type	Auto Design
S1.000	24.705	0.500	49.4	0.019	4.00	0.0	0.600	o	150	Pipe/Conduit	
S1.001	15.427	0.240	64.3	0.017	0.00	0.0	0.600	o	150	Pipe/Conduit	
S1.002	27.977	0.680	41.1	0.028	0.00	0.0	0.600	o	150	Pipe/Conduit	
S2.000	6.893	0.600	11.5	0.114	4.00	0.0	0.600	o	150	Pipe/Conduit	

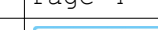
Network Results Table

PN	Rain (mm/hr)	T.C. (mins)	US/IL (m)	Σ I.Area (ha)	Σ Base Flow (l/s)	Foul (l/s)	Add Flow (l/s)	Vel (m/s)	Cap (l/s)	Flow (l/s)
S1.000	50.00	4.29	25.390	0.019	0.0	0.0	0.0	1.43	25.4	2.6
S1.001	50.00	4.49	24.890	0.036	0.0	0.0	0.0	1.26	22.2	4.9
S1.002	50.00	4.79	24.650	0.064	0.0	0.0	0.0	1.57	27.8	8.7
S2.000	50.00	4.04	25.450	0.114	0.0	0.0	0.0	2.99	52.8	15.4

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Network Design Table for Storm

Network Results Table

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Network Design Table for Storm

Network Results Table

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Network Design Table for Storm

PN	Length (m)	Fall (m)	Slope (1:X)	I.Area (ha)	T.E. (mins)	Base Flow (l/s)	k (mm)	HYD SECT	DIA (mm)	Section Type	Auto Design
S1.019	40.838	0.095	430.0	0.029	0.00	0.0	0.600	o	675	Pipe/Conduit	
S1.020	31.552	0.061	520.0	0.018	0.00	0.0	0.600	o	675	Pipe/Conduit	
S1.021	24.611	0.047	520.0	0.016	0.00	0.0	0.600	o	675	Pipe/Conduit	
S1.022	21.617	0.042	520.0	0.026	0.00	0.0	0.600	o	675	Pipe/Conduit	
S1.023	95.128	0.107	889.0	0.068	0.00	0.0	0.600	o	675	Pipe/Conduit	
S1.024	108.605	2.400	45.3	0.000	0.00	0.0	0.600	o	300	Pipe/Conduit	

Network Results Table

PN	Rain (mm/hr)	T.C. (mins)	US/IL (m)	Σ I.Area (ha)	Σ Base Flow (l/s)	Foul (l/s)	Add Flow (l/s)	Vel (m/s)	Cap (l/s)	Flow (l/s)
S1.019	50.00	8.83	19.552	2.204	0.0	0.0	0.0	1.26	450.0	298.4
S1.020	50.00	9.29	19.457	2.222	0.0	0.0	0.0	1.14	408.8	300.9
S1.021	50.00	9.65	19.396	2.238	0.0	0.0	0.0	1.14	408.8	303.1
S1.022	50.00	9.96	19.349	2.264	0.0	0.0	0.0	1.14	408.8	306.6
S1.023	50.00	11.78	19.307	2.332	0.0	0.0	0.0	0.87	311.6	315.8
S1.024	50.00	12.56	19.200	2.332	0.0	0.0	0.0	2.34	165.7	315.8

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Manhole Schedules for Storm

MH Name	MH CL (m)	MH Depth (m)	MH Connection	MH Diam., L*W (mm)	PN	Pipe Out Invert Level (m)	Pipe Out Diameter (mm)	PN	Pipes In Invert Level (m)	Pipes In Diameter (mm)	Backdrop (mm)
S1	26.740	1.350	Open Manhole	1200	S1.000	25.390	150				
S2	26.240	1.350	Open Manhole	1200	S1.001	24.890	150	S1.000	24.890	150	
S3	26.000	1.350	Open Manhole	1200	S1.002	24.650	150	S1.001	24.650	150	
S6	26.350	0.900	Open Manhole	1200	S2.000	25.450	150				
S7	26.200	1.350	Open Manhole	1200	S2.001	24.850	150	S2.000	24.850	150	
S8	25.750	1.350	Open Manhole	1200	S3.000	24.400	150				
S4	25.370	1.550	Open Manhole	1200	S1.003	23.820	300	S1.002	23.970	150	
								S2.001	23.970	150	
								S3.000	23.970	150	
S10	25.060	1.425	Open Manhole	1200	S4.000	23.635	225				
S11	24.950	0.800	Open Manhole	1200	S5.000	24.150	150				
S12	24.900	1.350	Open Manhole	1200	S5.001	23.550	150	S5.000	23.550	150	
S11	24.850	1.446	Open Manhole	1200	S4.001	23.404	225	S4.000	23.404	225	
								S5.001	23.479	150	
S14	24.934	1.350	Open Manhole	1200	S6.000	23.584	100				
S11	24.560	1.550	Open Manhole	1200	S4.002	23.010	300	S4.001	23.085	225	
								S6.000	23.210	100	
S5	24.420	1.651	Open Manhole	1350	S1.004	22.769	450	S1.003	22.919	300	
								S4.002	22.919	300	
S6	24.350	1.650	Open Manhole	1350	S1.005	22.700	450	S1.004	22.700	450	
S7	24.180	1.650	Open Manhole	1350	S1.006	22.530	450	S1.005	22.530	450	
S8	24.000	1.725	Open Manhole	1500	S1.007	22.275	525	S1.006	22.350	450	
S9	23.650	1.725	Open Manhole	1500	S1.008	21.925	525	S1.007	21.925	525	
S10	23.500	1.763	Open Manhole	1500	S1.009	21.737	525	S1.008	21.737	525	
S11	23.200	1.838	Open Manhole	1500	S1.010	21.362	600	S1.009	21.437	525	
S12	23.160	1.940	Open Manhole	1500	S1.011	21.220	600	S1.010	21.220	600	
S13	23.460	2.314	Open Manhole	1500	S1.012	21.146	600	S1.011	21.146	600	
S14	23.700	2.619	Open Manhole	1500	S1.013	21.081	600	S1.012	21.081	600	
S15	23.493	2.477	Open Manhole	1500	S1.014	21.017	600	S1.013	21.017	600	
S27	24.900	1.350	Open Manhole	1200	S7.000	23.550	150				
S28	24.840	1.830	Open Manhole	1200	S7.001	23.010	150	S7.000	23.010	150	
S29	25.100	1.350	Open Manhole	1200	S8.000	23.750	150				
S30	24.850	0.800	Open Manhole	1200	S9.000	24.050	150				
S31	24.800	1.350	Open Manhole	1200	S9.001	23.450	150	S9.000	23.450	150	
S30	24.750	1.425	Open Manhole	1200	S8.001	23.325	225	S8.000	23.400	150	
								S9.001	23.400	150	
S29	24.430	1.615	Open Manhole	1200	S7.002	22.815	225	S7.001	22.890	150	
								S8.001	22.815	225	

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Manhole Schedules for Storm

MH Name	MH CL (m)	MH Depth (m)	MH Connection	MH Diam., L*W (mm)	PN	Pipe Out Invert Level (m)	Diameter (mm)	PN	Pipes In Invert Level (m)	Diameter (mm)	Backdrop (mm)
S30	23.401	1.575	Open Manhole	1350	S7.003	21.826	375	S7.002	21.976	225	
S31	23.590	1.903	Open Manhole	1350	S7.004	21.687	375	S7.003	21.687	375	
S32	23.680	2.153	Open Manhole	1350	S7.005	21.527	375	S7.004	21.527	375	
S33	23.400	2.003	Open Manhole	1350	S7.006	21.397	375	S7.005	21.397	375	
S16	23.100	2.179	Open Manhole	1500	S1.015	20.921	600	S1.014	20.921	600	
								S7.006	21.146	375	
S17	22.650	1.900	Open Manhole	1500	S1.016	20.750	600	S1.015	20.750	600	
S18	22.150	1.800	Open Manhole	1500	S1.017	20.350	600	S1.016	20.350	600	
S19	21.790	1.799	Open Manhole	1500	S1.018	19.991	600	S1.017	19.991	600	
S42	22.700	0.800	Open Manhole	1200	S10.000	21.900	150				
S43	22.550	1.350	Open Manhole	1200	S10.001	21.200	150	S10.000	21.200	150	
S44	22.150	1.350	Open Manhole	1200	S10.002	20.800	150	S10.001	20.800	150	
S45	21.900	0.800	Open Manhole	1200	S11.000	21.100	150				
S46	21.850	1.350	Open Manhole	1200	S11.001	20.500	150	S11.000	20.500	150	
S47	21.900	0.800	Open Manhole	1200	S12.000	21.100	150				
S48	21.850	1.350	Open Manhole	1200	S12.001	20.500	150	S12.000	20.500	150	
S47	21.800	1.370	Open Manhole	1200	S11.002	20.430	150	S11.001	20.430	150	
								S12.001	20.430	150	
S48	22.000	1.754	Open Manhole	1200	S11.003	20.246	225	S11.002	20.321	150	
S45	21.910	1.800	Open Manhole	1200	S10.003	20.110	300	S10.002	20.260	150	
								S11.003	20.185	225	
S46	21.710	1.721	Open Manhole	1200	S10.004	19.989	300	S10.003	19.989	300	
S20	21.540	1.988	Open Manhole	1500	S1.019	19.552	675	S1.018	19.627	600	
								S10.004	19.927	300	
S21	21.660	2.203	Open Manhole	1500	S1.020	19.457	675	S1.019	19.457	675	
S22	21.600	2.204	Open Manhole	1500	S1.021	19.396	675	S1.020	19.396	675	
S23	21.350	2.001	Open Manhole	1500	S1.022	19.349	675	S1.021	19.349	675	
S24	21.232	1.925	Open Manhole	1500	S1.023	19.307	675	S1.022	19.307	675	
S25	20.100	0.900	Open Manhole	1500	S1.024	19.200	300	S1.023	19.200	675	
S	17.400	0.600	Open Manhole	0		OUTFALL		S1.024	16.800	300	

MH Name	Manhole Easting (m)	Manhole Northing (m)	Intersection Easting (m)	Intersection Northing (m)	Manhole Access	Layout (North)
S1	448931.014	116077.783	448931.014	116077.783	Required	

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Manhole Schedules for Storm

MH Name	Manhole Easting (m)	Manhole Northing (m)	Intersection Easting (m)	Intersection Northing (m)	Manhole Access	Layout (North)
S8	448882.348	116136.931	448882.348	116136.931	Required	
S9	448845.320	116136.832	448845.320	116136.832	Required	
S10	448832.171	116136.603	448832.171	116136.603	Required	
S11	448833.298	116157.091	448833.298	116157.091	Required	
S12	448834.137	116185.508	448834.137	116185.508	Required	
S13	448834.008	116215.225	448834.008	116215.225	Required	
S14	448837.897	116239.508	448837.897	116239.508	Required	
S15	448845.006	116262.330	448845.006	116262.330	Required	
S27	448953.713	116237.146	448953.713	116237.146	Required	
S28	448969.149	116264.724	448969.149	116264.724	Required	
S29	448903.662	116265.943	448903.662	116265.943	Required	
S30	448934.635	116263.654	448934.635	116263.654	Required	
S31	448937.576	116268.156	448937.576	116268.156	Required	
S30	448935.404	116274.655	448935.404	116274.655	Required	
S29	448962.215	116281.367	448962.215	116281.367	Required	

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PIPELINE SCHEDULES for Storm

Upstream Manhole

PN	Hyd Sect	Diam (mm)	MH Name	C.Level (m)	I.Level (m)	D.Depth (m)	MH Connection	MH DIAM., L*W (mm)
S1.000	o	150	S1	26.740	25.390	1.200	Open Manhole	1200
S1.001	o	150	S2	26.240	24.890	1.200	Open Manhole	1200
S1.002	o	150	S3	26.000	24.650	1.200	Open Manhole	1200
S2.000	o	150	S6	26.350	25.450	0.750	Open Manhole	1200
S2.001	o	150	S7	26.200	24.850	1.200	Open Manhole	1200
S3.000	o	150	S8	25.750	24.400	1.200	Open Manhole	1200
S1.003	o	300	S4	25.370	23.820	1.250	Open Manhole	1200
S4.000	o	225	S10	25.060	23.635	1.200	Open Manhole	1200
S5.000	o	150	S11	24.950	24.150	0.650	Open Manhole	1200
S5.001	o	150	S12	24.900	23.550	1.200	Open Manhole	1200
S4.001	o	225	S11	24.850	23.404	1.221	Open Manhole	1200
S6.000	o	100	S14	24.934	23.584	1.250	Open Manhole	1200
S4.002	o	300	S11	24.560	23.010	1.250	Open Manhole	1200

Downstream Manhole

PN	Length (m)	Slope (1:X)	MH Name	C.Level (m)	I.Level (m)	D.Depth (m)	MH Connection	MH DIAM., L*W (mm)
S1.000	24.705	49.4	S2	26.240	24.890	1.200	Open Manhole	1200
S1.001	15.427	64.3	S3	26.000	24.650	1.200	Open Manhole	1200
S1.002	27.977	41.1	S4	25.370	23.970	1.250	Open Manhole	1200
S2.000	6.893	11.5	S7	26.200	24.850	1.200	Open Manhole	1200
S2.001	19.226	21.8	S4	25.370	23.970	1.250	Open Manhole	1200
S3.000	32.098	74.6	S4	25.370	23.970	1.250	Open Manhole	1200
S1.003	26.440	29.3	S5	24.420	22.919	1.201	Open Manhole	1350
S4.000	21.932	94.9	S11	24.850	23.404	1.221	Open Manhole	1200
S5.000	8.227	13.7	S12	24.900	23.550	1.200	Open Manhole	1200
S5.001	10.622	149.6	S11	24.850	23.479	1.221	Open Manhole	1200
S4.001	27.932	87.6	S11	24.560	23.085	1.250	Open Manhole	1200
S6.000	34.804	93.1	S11	24.560	23.210	1.250	Open Manhole	1200
S4.002	15.195	167.0	S5	24.420	22.919	1.201	Open Manhole	1350

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PIPELINE SCHEDULES for Storm

Upstream Manhole

PN	Hyd Sect	Diam (mm)	MH Name	C.Level (m)	I.Level (m)	D.Depth (m)	MH Connection	MH DIAM., L*W (mm)
S1.004	o	450	S5	24.420	22.769	1.201	Open Manhole	1350
S1.005	o	450	S6	24.350	22.700	1.200	Open Manhole	1350
S1.006	o	450	S7	24.180	22.530	1.200	Open Manhole	1350
S1.007	o	525	S8	24.000	22.275	1.200	Open Manhole	1500
S1.008	o	525	S9	23.650	21.925	1.200	Open Manhole	1500
S1.009	o	525	S10	23.500	21.737	1.238	Open Manhole	1500
S1.010	o	600	S11	23.200	21.362	1.238	Open Manhole	1500
S1.011	o	600	S12	23.160	21.220	1.340	Open Manhole	1500
S1.012	o	600	S13	23.460	21.146	1.714	Open Manhole	1500
S1.013	o	600	S14	23.700	21.081	2.019	Open Manhole	1500
S1.014	o	600	S15	23.493	21.017	1.877	Open Manhole	1500
S7.000	o	150	S27	24.900	23.550	1.200	Open Manhole	1200
S7.001	o	150	S28	24.840	23.010	1.680	Open Manhole	1200
S8.000	o	150	S29	25.100	23.750	1.200	Open Manhole	1200
S9.000	o	150	S30	24.850	24.050	0.650	Open Manhole	1200
S9.001	o	150	S31	24.800	23.450	1.200	Open Manhole	1200

Downstream Manhole

PN	Length (m)	Slope (1:X)	MH Name	C.Level (m)	I.Level (m)	D.Depth (m)	MH Connection	MH DIAM., L*W (mm)
S1.004	8.061	116.8	S6	24.350	22.700	1.200	Open Manhole	1350
S1.005	17.287	101.7	S7	24.180	22.530	1.200	Open Manhole	1350
S1.006	16.284	90.5	S8	24.000	22.350	1.200	Open Manhole	1500
S1.007	37.029	105.8	S9	23.650	21.925	1.200	Open Manhole	1500
S1.008	13.150	70.0	S10	23.500	21.737	1.238	Open Manhole	1500
S1.009	20.519	68.4	S11	23.200	21.437	1.238	Open Manhole	1500
S1.010	28.429	200.2	S12	23.160	21.220	1.340	Open Manhole	1500
S1.011	29.718	400.0	S13	23.460	21.146	1.714	Open Manhole	1500
S1.012	24.592	380.0	S14	23.700	21.081	2.019	Open Manhole	1500
S1.013	23.904	370.0	S15	23.493	21.017	1.877	Open Manhole	1500
S1.014	24.742	258.8	S16	23.100	20.921	1.579	Open Manhole	1500
S7.000	31.603	58.5	S28	24.840	23.010	1.680	Open Manhole	1200
S7.001	18.030	150.3	S29	24.430	22.890	1.390	Open Manhole	1200
S8.000	32.916	94.0	S30	24.750	23.400	1.200	Open Manhole	1200
S9.000	5.378	9.0	S31	24.800	23.450	1.200	Open Manhole	1200
S9.001	6.852	137.0	S30	24.750	23.400	1.200	Open Manhole	1200

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PIPELINE SCHEDULES for Storm

Upstream Manhole

PN	Hyd Sect	Diam (mm)	MH Name	C.Level (m)	I.Level (m)	D.Depth (m)	MH Connection	MH DIAM., L*W (mm)
S8.001	o	225	S30	24.750	23.325	1.200	Open Manhole	1200
S7.002	o	225	S29	24.430	22.815	1.390	Open Manhole	1200
S7.003	o	375	S30	23.401	21.826	1.200	Open Manhole	1350
S7.004	o	375	S31	23.590	21.687	1.528	Open Manhole	1350
S7.005	o	375	S32	23.680	21.527	1.778	Open Manhole	1350
S7.006	o	375	S33	23.400	21.397	1.628	Open Manhole	1350
S1.015	o	600	S16	23.100	20.921	1.579	Open Manhole	1500
S1.016	o	600	S17	22.650	20.750	1.300	Open Manhole	1500
S1.017	o	600	S18	22.150	20.350	1.200	Open Manhole	1500
S1.018	o	600	S19	21.790	19.991	1.199	Open Manhole	1500
S10.000	o	150	S42	22.700	21.900	0.650	Open Manhole	1200
S10.001	o	150	S43	22.550	21.200	1.200	Open Manhole	1200
S10.002	o	150	S44	22.150	20.800	1.200	Open Manhole	1200
S11.000	o	150	S45	21.900	21.100	0.650	Open Manhole	1200
S11.001	o	150	S46	21.850	20.500	1.200	Open Manhole	1200
S12.000	o	150	S47	21.900	21.100	0.650	Open Manhole	1200

Downstream Manhole

PN	Length (m)	Slope (1:X)	MH Name	C.Level (m)	I.Level (m)	D.Depth (m)	MH Connection	MH DIAM., L*W (mm)
S8.001	27.638	54.2	S29	24.430	22.815	1.390	Open Manhole	1200
S7.002	38.852	46.3	S30	23.401	21.976	1.200	Open Manhole	1350
S7.003	23.133	166.4	S31	23.590	21.687	1.528	Open Manhole	1350
S7.004	38.011	238.0	S32	23.680	21.527	1.778	Open Manhole	1350
S7.005	31.029	238.0	S33	23.400	21.397	1.628	Open Manhole	1350
S7.006	12.853	51.2	S16	23.100	21.146	1.579	Open Manhole	1500
S1.015	34.210	200.0	S17	22.650	20.750	1.300	Open Manhole	1500
S1.016	35.912	89.8	S18	22.150	20.350	1.200	Open Manhole	1500
S1.017	32.324	90.0	S19	21.790	19.991	1.199	Open Manhole	1500
S1.018	24.558	67.5	S20	21.540	19.627	1.313	Open Manhole	1500
S10.000	11.822	16.9	S43	22.550	21.200	1.200	Open Manhole	1200
S10.001	37.337	93.3	S44	22.150	20.800	1.200	Open Manhole	1200
S10.002	29.760	55.1	S45	21.910	20.260	1.500	Open Manhole	1200
S11.000	5.120	8.5	S46	21.850	20.500	1.200	Open Manhole	1200
S11.001	10.428	149.0	S47	21.800	20.430	1.220	Open Manhole	1200
S12.000	5.178	8.6	S48	21.850	20.500	1.200	Open Manhole	1200

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PIPELINE SCHEDULES for Storm

Upstream Manhole

PN	Hyd Sect	Diam (mm)	MH Name	C.Level (m)	I.Level (m)	D.Depth (m)	MH Connection	MH DIAM., L*W (mm)
S12.001	o	150	S48	21.850	20.500	1.200	Open Manhole	1200
S11.002	o	150	S47	21.800	20.430	1.220	Open Manhole	1200
S11.003	o	225	S48	22.000	20.246	1.529	Open Manhole	1200
S10.003	o	300	S45	21.910	20.110	1.500	Open Manhole	1200
S10.004	o	300	S46	21.710	19.989	1.421	Open Manhole	1200
S1.019	o	675	S20	21.540	19.552	1.313	Open Manhole	1500
S1.020	o	675	S21	21.660	19.457	1.528	Open Manhole	1500
S1.021	o	675	S22	21.600	19.396	1.529	Open Manhole	1500
S1.022	o	675	S23	21.350	19.349	1.326	Open Manhole	1500
S1.023	o	675	S24	21.232	19.307	1.250	Open Manhole	1500
S1.024	o	300	S25	20.100	19.200	0.600	Open Manhole	1500

Downstream Manhole

PN	Length (m)	Slope (1:X)	MH Name	C.Level (m)	I.Level (m)	D.Depth (m)	MH Connection	MH DIAM., L*W (mm)
S12.001	7.268	103.8	S47	21.800	20.430	1.220	Open Manhole	1200
S11.002	16.418	150.0	S48	22.000	20.321	1.529	Open Manhole	1200
S11.003	9.150	150.0	S45	21.910	20.185	1.500	Open Manhole	1200
S10.003	20.118	167.0	S46	21.710	19.989	1.421	Open Manhole	1200
S10.004	14.743	238.0	S20	21.540	19.927	1.313	Open Manhole	1500
S1.019	40.838	430.0	S21	21.660	19.457	1.528	Open Manhole	1500
S1.020	31.552	520.0	S22	21.600	19.396	1.529	Open Manhole	1500
S1.021	24.611	520.0	S23	21.350	19.349	1.326	Open Manhole	1500
S1.022	21.617	520.0	S24	21.232	19.307	1.249	Open Manhole	1500
S1.023	95.128	889.0	S25	20.100	19.200	0.225	Open Manhole	1500
S1.024	108.605	45.3	S	17.400	16.800	0.300	Open Manhole	0

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Area Summary for Storm <table><tr><th>Pipe Number</th><th>PIMP Type</th><th>PIMP Name</th><th>PIMP (%)</th><th>Gross Area (ha)</th><th>Imp. Area (ha)</th><th>Pipe Total (ha)</th></tr><tr><td>1.000</td><td>-</td><td>-</td><td>100</td><td>0.019</td><td>0.019</td><td>0.019</td></tr><tr><td>1.001</td><td>-</td><td>-</td><td>100</td><td>0.017</td><td>0.017</td><td>0.017</td></tr><tr><td>1.002</td><td>-</td><td>-</td><td>100</td><td>0.028</td><td>0.028</td><td>0.028</td></tr><tr><td>2.000</td><td>-</td><td>-</td><td>100</td><td>0.114</td><td>0.114</td><td>0.114</td></tr><tr><td>2.001</td><td>-</td><td>-</td><td>100</td><td>0.000</td><td>0.000</td><td>0.000</td></tr><tr><td>3.000</td><td>-</td><td>-</td><td>100</td><td>0.032</td><td>0.032</td><td>0.032</td></tr><tr><td>1.003</td><td>-</td><td>-</td><td>100</td><td>0.133</td><td>0.133</td><td>0.133</td></tr><tr><td>4.000</td><td>-</td><td>-</td><td>100</td><td>0.085</td><td>0.085</td><td>0.085</td></tr><tr><td>5.000</td><td>-</td><td>-</td><td>100</td><td>0.084</td><td>0.084</td><td>0.084</td></tr><tr><td>5.001</td><td>-</td><td>-</td><td>100</td><td>0.000</td><td>0.000</td><td>0.000</td></tr><tr><td>4.001</td><td>-</td><td>-</td><td>100</td><td>0.000</td><td>0.000</td><td>0.000</td></tr><tr><td>6.000</td><td>-</td><td>-</td><td>100</td><td>0.012</td><td>0.012</td><td>0.012</td></tr><tr><td>4.002</td><td>-</td><td>-</td><td>100</td><td>0.075</td><td>0.075</td><td>0.075</td></tr><tr><td>1.004</td><td>-</td><td>-</td><td>100</td><td>0.000</td><td>0.000</td><td>0.000</td></tr><tr><td>1.005</td><td>-</td><td>-</td><td>100</td><td>0.037</td><td>0.037</td><td>0.037</td></tr><tr><td>1.006</td><td>-</td><td>-</td><td>100</td><td>0.000</td><td>0.000</td><td>0.000</td></tr><tr><td>1.007</td><td>-</td><td>-</td><td>100</td><td>0.358</td><td>0.358</td><td>0.358</td></tr><tr><td>1.008</td><td>-</td><td>-</td><td>100</td><td>0.040</td><td>0.040</td><td>0.040</td></tr><tr><td>1.009</td><td>-</td><td>-</td><td>100</td><td>0.107</td><td>0.107</td><td>0.107</td></tr><tr><td>1.010</td><td>-</td><td>-</td><td>100</td><td>0.027</td><td>0.027</td><td>0.027</td></tr><tr><td>1.011</td><td>-</td><td>-</td><td>100</td><td>0.018</td><td>0.018</td><td>0.018</td></tr><tr><td>1.012</td><td>-</td><td>-</td><td>100</td><td>0.024</td><td>0.024</td><td>0.024</td></tr><tr><td>1.013</td><td>-</td><td>-</td><td>100</td><td>0.024</td><td>0.024</td><td>0.024</td></tr><tr><td>1.014</td><td>-</td><td>-</td><td>100</td><td>0.024</td><td>0.024</td><td>0.024</td></tr><tr><td>7.000</td><td>-</td><td>-</td><td>100</td><td>0.027</td><td>0.027</td><td>0.027</td></tr><tr><td>7.001</td><td>-</td><td>-</td><td>100</td><td>0.044</td><td>0.044</td><td>0.044</td></tr><tr><td>8.000</td><td>-</td><td>-</td><td>100</td><td>0.032</td><td>0.032</td><td>0.032</td></tr><tr><td>9.000</td><td>-</td><td>-</td><td>100</td><td>0.041</td><td>0.041</td><td>0.041</td></tr><tr><td>9.001</td><td>-</td><td>-</td><td>100</td><td>0.000</td><td>0.000</td><td>0.000</td></tr><tr><td>8.001</td><td>-</td><td>-</td><td>100</td><td>0.066</td><td>0.066</td><td>0.066</td></tr><tr><td>7.002</td><td>-</td><td>-</td><td>100</td><td>0.031</td><td>0.031</td><td>0.031</td></tr><tr><td>7.003</td><td>-</td><td>-</td><td>100</td><td>0.000</td><td>0.000</td><td>0.000</td></tr><tr><td>7.004</td><td>-</td><td>-</td><td>100</td><td>0.101</td><td>0.101</td><td>0.101</td></tr><tr><td>7.005</td><td>-</td><td>-</td><td>100</td><td>0.076</td><td>0.076</td><td>0.076</td></tr><tr><td>7.006</td><td>-</td><td>-</td><td>100</td><td>0.080</td><td>0.080</td><td>0.080</td></tr><tr><td>1.015</td><td>-</td><td>-</td><td>100</td><td>0.000</td><td>0.000</td><td>0.000</td></tr><tr><td>1.016</td><td>-</td><td>-</td><td>100</td><td>0.000</td><td>0.000</td><td>0.000</td></tr><tr><td>1.017</td><td>-</td><td>-</td><td>100</td><td>0.030</td><td>0.030</td><td>0.030</td></tr><tr><td>1.018</td><td>-</td><td>-</td><td>100</td><td>0.020</td><td>0.020</td><td>0.020</td></tr><tr><td>10.000</td><td>-</td><td>-</td><td>100</td><td>0.111</td><td>0.111</td><td>0.111</td></tr><tr><td>10.001</td><td>-</td><td>-</td><td>100</td><td>0.000</td><td>0.000</td><td>0.000</td></tr><tr><td>10.002</td><td>-</td><td>-</td><td>100</td><td>0.064</td><td>0.064</td><td>0.064</td></tr><tr><td>11.000</td><td>-</td><td>-</td><td>100</td><td>0.059</td><td>0.059</td><td>0.059</td></tr><tr><td>11.001</td><td>-</td><td>-</td><td>100</td><td>0.000</td><td>0.000</td><td>0.000</td></tr><tr><td>12.000</td><td>-</td><td>-</td><td>100</td><td>0.044</td><td>0.044</td><td>0.044</td></tr><tr><td>12.001</td><td>-</td><td>-</td><td>100</td><td>0.000</td><td>0.000</td><td>0.000</td></tr><tr><td>11.002</td><td>-</td><td>-</td><td>100</td><td>0.000</td><td>0.000</td><td>0.000</td></tr><tr><td>11.003</td><td>-</td><td>-</td><td>100</td><td>0.026</td><td>0.026</td><td>0.026</td></tr><tr><td>10.003</td><td>-</td><td>-</td><td>100</td><td>0.033</td><td>0.033</td><td>0.033</td></tr><tr><td>10.004</td><td>-</td><td>-</td><td>100</td><td>0.032</td><td>0.032</td><td>0.032</td></tr><tr><td>1.019</td><td>-</td><td>-</td><td>100</td><td>0.029</td><td>0.029</td><td>0.029</td></tr></table>			Pipe Number	PIMP Type	PIMP Name	PIMP (%)	Gross Area (ha)	Imp. Area (ha)	Pipe Total (ha)	1.000	-	-	100	0.019	0.019	0.019	1.001	-	-	100	0.017	0.017	0.017	1.002	-	-	100	0.028	0.028	0.028	2.000	-	-	100	0.114	0.114	0.114	2.001	-	-	100	0.000	0.000	0.000	3.000	-	-	100	0.032	0.032	0.032	1.003	-	-	100	0.133	0.133	0.133	4.000	-	-	100	0.085	0.085	0.085	5.000	-	-	100	0.084	0.084	0.084	5.001	-	-	100	0.000	0.000	0.000	4.001	-	-	100	0.000	0.000	0.000	6.000	-	-	100	0.012	0.012	0.012	4.002	-	-	100	0.075	0.075	0.075	1.004	-	-	100	0.000	0.000	0.000	1.005	-	-	100	0.037	0.037	0.037	1.006	-	-	100	0.000	0.000	0.000	1.007	-	-	100	0.358	0.358	0.358	1.008	-	-	100	0.040	0.040	0.040	1.009	-	-	100	0.107	0.107	0.107	1.010	-	-	100	0.027	0.027	0.027	1.011	-	-	100	0.018	0.018	0.018	1.012	-	-	100	0.024	0.024	0.024	1.013	-	-	100	0.024	0.024	0.024	1.014	-	-	100	0.024	0.024	0.024	7.000	-	-	100	0.027	0.027	0.027	7.001	-	-	100	0.044	0.044	0.044	8.000	-	-	100	0.032	0.032	0.032	9.000	-	-	100	0.041	0.041	0.041	9.001	-	-	100	0.000	0.000	0.000	8.001	-	-	100	0.066	0.066	0.066	7.002	-	-	100	0.031	0.031	0.031	7.003	-	-	100	0.000	0.000	0.000	7.004	-	-	100	0.101	0.101	0.101	7.005	-	-	100	0.076	0.076	0.076	7.006	-	-	100	0.080	0.080	0.080	1.015	-	-	100	0.000	0.000	0.000	1.016	-	-	100	0.000	0.000	0.000	1.017	-	-	100	0.030	0.030	0.030	1.018	-	-	100	0.020	0.020	0.020	10.000	-	-	100	0.111	0.111	0.111	10.001	-	-	100	0.000	0.000	0.000	10.002	-	-	100	0.064	0.064	0.064	11.000	-	-	100	0.059	0.059	0.059	11.001	-	-	100	0.000	0.000	0.000	12.000	-	-	100	0.044	0.044	0.044	12.001	-	-	100	0.000	0.000	0.000	11.002	-	-	100	0.000	0.000	0.000	11.003	-	-	100	0.026	0.026	0.026	10.003	-	-	100	0.033	0.033	0.033	10.004	-	-	100	0.032	0.032	0.032	1.019	-	-	100	0.029	0.029	0.029
Pipe Number	PIMP Type	PIMP Name	PIMP (%)	Gross Area (ha)	Imp. Area (ha)	Pipe Total (ha)																																																																																																																																																																																																																																																																																																																																																																								
1.000	-	-	100	0.019	0.019	0.019																																																																																																																																																																																																																																																																																																																																																																								
1.001	-	-	100	0.017	0.017	0.017																																																																																																																																																																																																																																																																																																																																																																								
1.002	-	-	100	0.028	0.028	0.028																																																																																																																																																																																																																																																																																																																																																																								
2.000	-	-	100	0.114	0.114	0.114																																																																																																																																																																																																																																																																																																																																																																								
2.001	-	-	100	0.000	0.000	0.000																																																																																																																																																																																																																																																																																																																																																																								
3.000	-	-	100	0.032	0.032	0.032																																																																																																																																																																																																																																																																																																																																																																								
1.003	-	-	100	0.133	0.133	0.133																																																																																																																																																																																																																																																																																																																																																																								
4.000	-	-	100	0.085	0.085	0.085																																																																																																																																																																																																																																																																																																																																																																								
5.000	-	-	100	0.084	0.084	0.084																																																																																																																																																																																																																																																																																																																																																																								
5.001	-	-	100	0.000	0.000	0.000																																																																																																																																																																																																																																																																																																																																																																								
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6.000	-	-	100	0.012	0.012	0.012																																																																																																																																																																																																																																																																																																																																																																								
4.002	-	-	100	0.075	0.075	0.075																																																																																																																																																																																																																																																																																																																																																																								
1.004	-	-	100	0.000	0.000	0.000																																																																																																																																																																																																																																																																																																																																																																								
1.005	-	-	100	0.037	0.037	0.037																																																																																																																																																																																																																																																																																																																																																																								
1.006	-	-	100	0.000	0.000	0.000																																																																																																																																																																																																																																																																																																																																																																								
1.007	-	-	100	0.358	0.358	0.358																																																																																																																																																																																																																																																																																																																																																																								
1.008	-	-	100	0.040	0.040	0.040																																																																																																																																																																																																																																																																																																																																																																								
1.009	-	-	100	0.107	0.107	0.107																																																																																																																																																																																																																																																																																																																																																																								
1.010	-	-	100	0.027	0.027	0.027																																																																																																																																																																																																																																																																																																																																																																								
1.011	-	-	100	0.018	0.018	0.018																																																																																																																																																																																																																																																																																																																																																																								
1.012	-	-	100	0.024	0.024	0.024																																																																																																																																																																																																																																																																																																																																																																								
1.013	-	-	100	0.024	0.024	0.024																																																																																																																																																																																																																																																																																																																																																																								
1.014	-	-	100	0.024	0.024	0.024																																																																																																																																																																																																																																																																																																																																																																								
7.000	-	-	100	0.027	0.027	0.027																																																																																																																																																																																																																																																																																																																																																																								
7.001	-	-	100	0.044	0.044	0.044																																																																																																																																																																																																																																																																																																																																																																								
8.000	-	-	100	0.032	0.032	0.032																																																																																																																																																																																																																																																																																																																																																																								
9.000	-	-	100	0.041	0.041	0.041																																																																																																																																																																																																																																																																																																																																																																								
9.001	-	-	100	0.000	0.000	0.000																																																																																																																																																																																																																																																																																																																																																																								
8.001	-	-	100	0.066	0.066	0.066																																																																																																																																																																																																																																																																																																																																																																								
7.002	-	-	100	0.031	0.031	0.031																																																																																																																																																																																																																																																																																																																																																																								
7.003	-	-	100	0.000	0.000	0.000																																																																																																																																																																																																																																																																																																																																																																								
7.004	-	-	100	0.101	0.101	0.101																																																																																																																																																																																																																																																																																																																																																																								
7.005	-	-	100	0.076	0.076	0.076																																																																																																																																																																																																																																																																																																																																																																								
7.006	-	-	100	0.080	0.080	0.080																																																																																																																																																																																																																																																																																																																																																																								
1.015	-	-	100	0.000	0.000	0.000																																																																																																																																																																																																																																																																																																																																																																								
1.016	-	-	100	0.000	0.000	0.000																																																																																																																																																																																																																																																																																																																																																																								
1.017	-	-	100	0.030	0.030	0.030																																																																																																																																																																																																																																																																																																																																																																								
1.018	-	-	100	0.020	0.020	0.020																																																																																																																																																																																																																																																																																																																																																																								
10.000	-	-	100	0.111	0.111	0.111																																																																																																																																																																																																																																																																																																																																																																								
10.001	-	-	100	0.000	0.000	0.000																																																																																																																																																																																																																																																																																																																																																																								
10.002	-	-	100	0.064	0.064	0.064																																																																																																																																																																																																																																																																																																																																																																								
11.000	-	-	100	0.059	0.059	0.059																																																																																																																																																																																																																																																																																																																																																																								
11.001	-	-	100	0.000	0.000	0.000																																																																																																																																																																																																																																																																																																																																																																								
12.000	-	-	100	0.044	0.044	0.044																																																																																																																																																																																																																																																																																																																																																																								
12.001	-	-	100	0.000	0.000	0.000																																																																																																																																																																																																																																																																																																																																																																								
11.002	-	-	100	0.000	0.000	0.000																																																																																																																																																																																																																																																																																																																																																																								
11.003	-	-	100	0.026	0.026	0.026																																																																																																																																																																																																																																																																																																																																																																								
10.003	-	-	100	0.033	0.033	0.033																																																																																																																																																																																																																																																																																																																																																																								
10.004	-	-	100	0.032	0.032	0.032																																																																																																																																																																																																																																																																																																																																																																								
1.019	-	-	100	0.029	0.029	0.029																																																																																																																																																																																																																																																																																																																																																																								
©1982-2020 Innovyze																																																																																																																																																																																																																																																																																																																																																																														

Odyssey Markides LLP		Page 17
Tuscany House White Hart Lane Basingstoke RG21 4AF	ONE HORTON HEATH BASIN L	
Date 15/12/2020 12:27 File 2020-11-03 Basin South ...	Designed by NA Checked by GG	
Innovyze	Network 2020.1	

Area Summary for Storm

Pipe Number	PIMP Type	PIMP Name	PIMP (%)	Gross Area (ha)	Imp. Area (ha)	Pipe Total (ha)
1.020	-	-	100	0.018	0.018	0.018
1.021	-	-	100	0.016	0.016	0.016
1.022	-	-	100	0.026	0.026	0.026
1.023	-	-	100	0.068	0.068	0.068
1.024	-	-	100	0.000	0.000	0.000
				Total	Total	Total
				2.332	2.332	2.332

Free Flowing Outfall Details for Storm

Outfall Pipe Number	Outfall Name	C. Level (m)	I. Level (m)	Min I. Level (m)	D,L (mm)	W (mm)
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S1.024	S	17.400	16.800	0.000	0	0
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Simulation Criteria for Storm

Volumetric Runoff Coeff	0.750	Additional Flow - % of Total Flow	0.000
Areal Reduction Factor	1.000	MADD Factor * 10m ³ /ha Storage	2.000
Hot Start (mins)	0	Inlet Coefficient	0.800
Hot Start Level (mm)	0	Flow per Person per Day (l/per/day)	0.000
Manhole Headloss Coeff (Global)	0.500	Run Time (mins)	60
Foul Sewage per hectare (l/s)	0.000	Output Interval (mins)	1

Number of Input Hydrographs	0	Number of Storage Structures	7
Number of Online Controls	7	Number of Time/Area Diagrams	0
Number of Offline Controls	0	Number of Real Time Controls	0

Synthetic Rainfall Details

Rainfall Model	FSR	Profile Type	Summer
Return Period (years)	100	Cv (Summer)	0.750
Region	England and Wales	Cv (Winter)	0.840
M5-60 (mm)	19.900	Storm Duration (mins)	30
Ratio R	0.350		

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Online Controls for Storm

Hydro-Brake® Optimum Manhole: S7, DS/PN: S2.001, Volume (m³): 1.6

Unit Reference MD-SHE-0146-1050-1200-1050
Design Head (m) 1.200
Design Flow (l/s) 10.5
Flush-Flo™ Calculated
Objective Minimise upstream storage
Application Surface
Sump Available Yes
Diameter (mm) 146
Invert Level (m) 24.850
Minimum Outlet Pipe Diameter (mm) 225
Suggested Manhole Diameter (mm) 1200

Control Points	Head (m)	Flow (l/s)
Design Point (Calculated)	1.200	10.5
Flush-Flo™	0.359	10.5
Kick-Flo®	0.783	8.6
Mean Flow over Head Range	-	9.1

The hydrological calculations have been based on the Head/Discharge relationship for the Hydro-Brake® Optimum as specified. Should another type of control device other than a Hydro-Brake Optimum® be utilised then these storage routing calculations will be invalidated

Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)
0.100	5.3	1.200	10.5	3.000	16.2	7.000	24.3
0.200	9.9	1.400	11.3	3.500	17.4	7.500	25.1
0.300	10.4	1.600	12.0	4.000	18.6	8.000	25.9
0.400	10.5	1.800	12.7	4.500	19.7	8.500	26.7
0.500	10.3	2.000	13.4	5.000	20.7	9.000	27.4
0.600	10.0	2.200	14.0	5.500	21.7	9.500	28.2
0.800	8.7	2.400	14.6	6.000	22.6		
1.000	9.6	2.600	15.1	6.500	23.5		

Hydro-Brake® Optimum Manhole: S12, DS/PN: S5.001, Volume (m³): 1.7

Unit Reference MD-SHE-0099-4500-1100-4500
Design Head (m) 1.100
Design Flow (l/s) 4.5
Flush-Flo™ Calculated
Objective Minimise upstream storage
Application Surface
Sump Available Yes
Diameter (mm) 99
Invert Level (m) 23.550
Minimum Outlet Pipe Diameter (mm) 150
Suggested Manhole Diameter (mm) 1200

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Hydro-Brake® Optimum Manhole: S12, DS/PN: S5.001, Volume (m³): 1.7

Control Points	Head (m)	Flow (l/s)
Design Point (Calculated)	1.100	4.5
Flush-Flo™	0.329	4.5
Kick-Flo®	0.693	3.6
Mean Flow over Head Range	-	3.9

The hydrological calculations have been based on the Head/Discharge relationship for the Hydro-Brake® Optimum as specified. Should another type of control device other than a Hydro-Brake Optimum® be utilised then these storage routing calculations will be invalidated

Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)
0.100	3.2	1.200	4.7	3.000	7.2	7.000	10.7
0.200	4.3	1.400	5.0	3.500	7.7	7.500	11.1
0.300	4.5	1.600	5.4	4.000	8.2	8.000	11.4
0.400	4.5	1.800	5.7	4.500	8.7	8.500	11.8
0.500	4.4	2.000	5.9	5.000	9.1	9.000	12.1
0.600	4.1	2.200	6.2	5.500	9.6	9.500	12.4
0.800	3.9	2.400	6.5	6.000	10.0		
1.000	4.3	2.600	6.7	6.500	10.4		

Hydro-Brake® Optimum Manhole: S31, DS/PN: S9.001, Volume (m³): 1.6

Unit Reference	MD-SHE-0093-4000-1100-4000
Design Head (m)	1.100
Design Flow (l/s)	4.0
Flush-Flo™	Calculated
Objective	Minimise upstream storage
Application	Surface
Sump Available	Yes
Diameter (mm)	93
Invert Level (m)	23.450
Minimum Outlet Pipe Diameter (mm)	150
Suggested Manhole Diameter (mm)	1200

Control Points	Head (m)	Flow (l/s)
Design Point (Calculated)	1.100	4.0
Flush-Flo™	0.326	4.0
Kick-Flo®	0.684	3.2
Mean Flow over Head Range	-	3.5

The hydrological calculations have been based on the Head/Discharge relationship for the Hydro-Brake® Optimum as specified. Should another type of control device other than a Hydro-Brake Optimum® be utilised then these storage routing calculations will be invalidated

Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)
0.100	3.0	0.300	4.0	0.500	3.9	0.800	3.5
0.200	3.8	0.400	4.0	0.600	3.6	1.000	3.8

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<u>Hydro-Brake® Optimum Manhole: S31, DS/PN: S9.001, Volume (m³): 1.6</u>							
Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)
1.200	4.2	2.400	5.7	5.000	8.1	8.000	10.2
1.400	4.5	2.600	6.0	5.500	8.5	8.500	10.5
1.600	4.8	3.000	6.4	6.000	8.9	9.000	10.7
1.800	5.0	3.500	6.9	6.500	9.2	9.500	11.0
2.000	5.3	4.000	7.3	7.000	9.5		
2.200	5.5	4.500	7.7	7.500	9.8		
<u>Hydro-Brake® Optimum Manhole: S43, DS/PN: S10.001, Volume (m³): 1.7</u>							
Unit Reference MD-SHE-0079-3000-1200-3000							
Design Head (m) 1.200							
Design Flow (l/s) 3.0							
Flush-Flo™ Calculated							
Objective Minimise upstream storage							
Application Surface							
Sump Available Yes							
Diameter (mm) 79							
Invert Level (m) 21.200							
Minimum Outlet Pipe Diameter (mm) 100							
Suggested Manhole Diameter (mm) 1200							
Control Points Head (m) Flow (l/s)							
Design Point (Calculated) 1.200 3.0							
Flush-Flo™ 0.348 2.9							
Kick-Flo® 0.707 2.4							
Mean Flow over Head Range - 2.6							
The hydrological calculations have been based on the Head/Discharge relationship for the Hydro-Brake® Optimum as specified. Should another type of control device other than a Hydro-Brake Optimum® be utilised then these storage routing calculations will be invalidated							
Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)
0.100	2.3	1.200	3.0	3.000	4.6	7.000	6.8
0.200	2.8	1.400	3.2	3.500	4.9	7.500	7.0
0.300	2.9	1.600	3.4	4.000	5.2	8.000	7.3
0.400	2.9	1.800	3.6	4.500	5.5	8.500	7.5
0.500	2.8	2.000	3.8	5.000	5.8	9.000	7.7
0.600	2.7	2.200	4.0	5.500	6.1	9.500	7.9
0.800	2.5	2.400	4.1	6.000	6.3		
1.000	2.8	2.600	4.3	6.500	6.6		
<u>Hydro-Brake® Optimum Manhole: S46, DS/PN: S11.001, Volume (m³): 1.6</u>							
Unit Reference MD-SHE-0081-3000-1100-3000							
Design Head (m) 1.100							
Design Flow (l/s) 3.0							
Flush-Flo™ Calculated							
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<u>Hydro-Brake® Optimum Manhole: S46, DS/PN: S11.001, Volume (m³): 1.6</u> Objective Minimise upstream storage Application Surface Sump Available Yes Diameter (mm) 81 Invert Level (m) 20.500 Minimum Outlet Pipe Diameter (mm) 100 Suggested Manhole Diameter (mm) 1200 <table><tr><th>Control Points</th><th>Head (m)</th><th>Flow (l/s)</th></tr><tr><td>Design Point (Calculated)</td><td>1.100</td><td>3.0</td></tr><tr><td>Flush-Flo™</td><td>0.333</td><td>3.0</td></tr><tr><td>Kick-Flo®</td><td>0.682</td><td>2.4</td></tr><tr><td>Mean Flow over Head Range</td><td>-</td><td>2.6</td></tr></table> The hydrological calculations have been based on the Head/Discharge relationship for the Hydro-Brake® Optimum as specified. Should another type of control device other than a Hydro-Brake Optimum® be utilised then these storage routing calculations will be invalidated <table><tr><th>Depth (m)</th><th>Flow (l/s)</th><th>Depth (m)</th><th>Flow (l/s)</th><th>Depth (m)</th><th>Flow (l/s)</th><th>Depth (m)</th><th>Flow (l/s)</th></tr><tr><td>0.100</td><td>2.4</td><td>1.200</td><td>3.1</td><td>3.000</td><td>4.8</td><td>7.000</td><td>7.1</td></tr><tr><td>0.200</td><td>2.9</td><td>1.400</td><td>3.4</td><td>3.500</td><td>5.1</td><td>7.500</td><td>7.3</td></tr><tr><td>0.300</td><td>3.0</td><td>1.600</td><td>3.6</td><td>4.000</td><td>5.5</td><td>8.000</td><td>7.6</td></tr><tr><td>0.400</td><td>3.0</td><td>1.800</td><td>3.8</td><td>4.500</td><td>5.8</td><td>8.500</td><td>7.8</td></tr><tr><td>0.500</td><td>2.9</td><td>2.000</td><td>4.0</td><td>5.000</td><td>6.1</td><td>9.000</td><td>8.0</td></tr><tr><td>0.600</td><td>2.7</td><td>2.200</td><td>4.1</td><td>5.500</td><td>6.3</td><td>9.500</td><td>8.2</td></tr><tr><td>0.800</td><td>2.6</td><td>2.400</td><td>4.3</td><td>6.000</td><td>6.6</td><td></td><td></td></tr><tr><td>1.000</td><td>2.9</td><td>2.600</td><td>4.5</td><td>6.500</td><td>6.9</td><td></td><td></td></tr></table> <u>Hydro-Brake® Optimum Manhole: S48, DS/PN: S12.001, Volume (m³): 1.6</u> Unit Reference MD-SHE-0081-3000-1100-3000 Design Head (m) 1.100 Design Flow (l/s) 3.0 Flush-Flo™ Calculated Objective Minimise upstream storage Application Surface Sump Available Yes Diameter (mm) 81 Invert Level (m) 20.500 Minimum Outlet Pipe Diameter (mm) 100 Suggested Manhole Diameter (mm) 1200 <table><tr><th>Control Points</th><th>Head (m)</th><th>Flow (l/s)</th></tr><tr><td>Design Point (Calculated)</td><td>1.100</td><td>3.0</td></tr><tr><td>Flush-Flo™</td><td>0.333</td><td>3.0</td></tr><tr><td>Kick-Flo®</td><td>0.682</td><td>2.4</td></tr><tr><td>Mean Flow over Head Range</td><td>-</td><td>2.6</td></tr></table> The hydrological calculations have been based on the Head/Discharge relationship for the Hydro-Brake® Optimum as specified. Should another type of control device other than a			Control Points	Head (m)	Flow (l/s)	Design Point (Calculated)	1.100	3.0	Flush-Flo™	0.333	3.0	Kick-Flo®	0.682	2.4	Mean Flow over Head Range	-	2.6	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	0.100	2.4	1.200	3.1	3.000	4.8	7.000	7.1	0.200	2.9	1.400	3.4	3.500	5.1	7.500	7.3	0.300	3.0	1.600	3.6	4.000	5.5	8.000	7.6	0.400	3.0	1.800	3.8	4.500	5.8	8.500	7.8	0.500	2.9	2.000	4.0	5.000	6.1	9.000	8.0	0.600	2.7	2.200	4.1	5.500	6.3	9.500	8.2	0.800	2.6	2.400	4.3	6.000	6.6			1.000	2.9	2.600	4.5	6.500	6.9			Control Points	Head (m)	Flow (l/s)	Design Point (Calculated)	1.100	3.0	Flush-Flo™	0.333	3.0	Kick-Flo®	0.682	2.4	Mean Flow over Head Range	-	2.6
Control Points	Head (m)	Flow (l/s)																																																																																																						
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0.100	2.4	1.200	3.1	3.000	4.8	7.000	7.1																																																																																																	
0.200	2.9	1.400	3.4	3.500	5.1	7.500	7.3																																																																																																	
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Mean Flow over Head Range	-	2.6																																																																																																						
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Hydro-Brake® Optimum Manhole: S48, DS/PN: S12.001, Volume (m³): 1.6

Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)
0.100	2.4	1.200	3.1	3.000	4.8	7.000	7.1
0.200	2.9	1.400	3.4	3.500	5.1	7.500	7.3
0.300	3.0	1.600	3.6	4.000	5.5	8.000	7.6
0.400	3.0	1.800	3.8	4.500	5.8	8.500	7.8
0.500	2.9	2.000	4.0	5.000	6.1	9.000	8.0
0.600	2.7	2.200	4.1	5.500	6.3	9.500	8.2
0.800	2.6	2.400	4.3	6.000	6.6		
1.000	2.9	2.600	4.5	6.500	6.9		

Unit Reference	MD-SHE-0268-3960-0900-3960
Design Head (m)	0.900
Design Flow (l/s)	39.6
Flush-Flo™	Calculated
Objective	Minimise upstream storage
Application	Surface
Sump Available	Yes
Diameter (mm)	268
Invert Level (m)	19.200
Minimum Outlet Pipe Diameter (mm)	300
Suggested Manhole Diameter (mm)	1800

Design Point (Calculated)	0.900	39.6
Flush-Flo™	0.405	39.6
Kick-Flo®	0.709	35.3
Mean Flow over Head Range	-	31.7

Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)
0.100	8.5	1.200	45.5	3.000	70.8	7.000	107.0
0.200	27.5	1.400	49.0	3.500	76.3	7.500	110.7
0.300	38.9	1.600	52.2	4.000	81.5	8.000	114.2
0.400	39.6	1.800	55.3	4.500	86.3	8.500	117.7
0.500	39.2	2.000	58.2	5.000	90.8	9.000	121.0
0.600	38.0	2.200	60.9	5.500	95.1	9.500	123.5
0.800	37.4	2.400	63.6	6.000	99.3		
1.000	41.7	2.600	66.1	6.500	103.2		

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Storage Structures for Storm Porous Car Park Manhole: S6, DS/PN: S2.000 <table><tr><td>Infiltration Coefficient Base (m/hr)</td><td>0.00000</td><td>Width (m)</td><td>10.3</td></tr><tr><td>Membrane Percolation (mm/hr)</td><td>1000</td><td>Length (m)</td><td>37.1</td></tr><tr><td>Max Percolation (l/s)</td><td>106.1</td><td>Slope (1:X)</td><td>40.0</td></tr><tr><td>Safety Factor</td><td>2.0</td><td>Depression Storage (mm)</td><td>5</td></tr><tr><td>Porosity</td><td>0.30</td><td>Evaporation (mm/day)</td><td>3</td></tr><tr><td>Invert Level (m)</td><td>25.450</td><td>Membrane Depth (mm)</td><td>0</td></tr></table> Porous Car Park Manhole: S11, DS/PN: S5.000 <table><tr><td>Infiltration Coefficient Base (m/hr)</td><td>0.00000</td><td>Width (m)</td><td>12.0</td></tr><tr><td>Membrane Percolation (mm/hr)</td><td>1000</td><td>Length (m)</td><td>59.8</td></tr><tr><td>Max Percolation (l/s)</td><td>199.3</td><td>Slope (1:X)</td><td>50.0</td></tr><tr><td>Safety Factor</td><td>2.0</td><td>Depression Storage (mm)</td><td>5</td></tr><tr><td>Porosity</td><td>0.30</td><td>Evaporation (mm/day)</td><td>3</td></tr><tr><td>Invert Level (m)</td><td>24.150</td><td>Membrane Depth (mm)</td><td>0</td></tr></table> Porous Car Park Manhole: S30, DS/PN: S9.000 <table><tr><td>Infiltration Coefficient Base (m/hr)</td><td>0.00000</td><td>Width (m)</td><td>12.0</td></tr><tr><td>Membrane Percolation (mm/hr)</td><td>1000</td><td>Length (m)</td><td>59.9</td></tr><tr><td>Max Percolation (l/s)</td><td>199.7</td><td>Slope (1:X)</td><td>50.0</td></tr><tr><td>Safety Factor</td><td>2.0</td><td>Depression Storage (mm)</td><td>5</td></tr><tr><td>Porosity</td><td>0.30</td><td>Evaporation (mm/day)</td><td>3</td></tr><tr><td>Invert Level (m)</td><td>24.050</td><td>Membrane Depth (mm)</td><td>0</td></tr></table> Porous Car Park Manhole: S42, DS/PN: S10.000 <table><tr><td>Infiltration Coefficient Base (m/hr)</td><td>0.00000</td><td>Width (m)</td><td>15.8</td></tr><tr><td>Membrane Percolation (mm/hr)</td><td>1000</td><td>Length (m)</td><td>29.0</td></tr><tr><td>Max Percolation (l/s)</td><td>127.3</td><td>Slope (1:X)</td><td>80.0</td></tr><tr><td>Safety Factor</td><td>2.0</td><td>Depression Storage (mm)</td><td>5</td></tr><tr><td>Porosity</td><td>0.30</td><td>Evaporation (mm/day)</td><td>3</td></tr><tr><td>Invert Level (m)</td><td>21.900</td><td>Membrane Depth (mm)</td><td>0</td></tr></table> Porous Car Park Manhole: S45, DS/PN: S11.000 <table><tr><td>Infiltration Coefficient Base (m/hr)</td><td>0.00000</td><td>Width (m)</td><td>12.8</td></tr><tr><td>Membrane Percolation (mm/hr)</td><td>1000</td><td>Length (m)</td><td>44.5</td></tr><tr><td>Max Percolation (l/s)</td><td>158.2</td><td>Slope (1:X)</td><td>50.0</td></tr><tr><td>Safety Factor</td><td>2.0</td><td>Depression Storage (mm)</td><td>5</td></tr><tr><td>Porosity</td><td>0.30</td><td>Evaporation (mm/day)</td><td>3</td></tr><tr><td>Invert Level (m)</td><td>21.100</td><td>Membrane Depth (mm)</td><td>0</td></tr></table> Porous Car Park Manhole: S47, DS/PN: S12.000 <table><tr><td>Infiltration Coefficient Base (m/hr)</td><td>0.00000</td><td>Max Percolation (l/s)</td><td>81.8</td></tr><tr><td>Membrane Percolation (mm/hr)</td><td>1000</td><td>Safety Factor</td><td>2.0</td></tr></table>			Infiltration Coefficient Base (m/hr)	0.00000	Width (m)	10.3	Membrane Percolation (mm/hr)	1000	Length (m)	37.1	Max Percolation (l/s)	106.1	Slope (1:X)	40.0	Safety Factor	2.0	Depression Storage (mm)	5	Porosity	0.30	Evaporation (mm/day)	3	Invert Level (m)	25.450	Membrane Depth (mm)	0	Infiltration Coefficient Base (m/hr)	0.00000	Width (m)	12.0	Membrane Percolation (mm/hr)	1000	Length (m)	59.8	Max Percolation (l/s)	199.3	Slope (1:X)	50.0	Safety Factor	2.0	Depression Storage (mm)	5	Porosity	0.30	Evaporation (mm/day)	3	Invert Level (m)	24.150	Membrane Depth (mm)	0	Infiltration Coefficient Base (m/hr)	0.00000	Width (m)	12.0	Membrane Percolation (mm/hr)	1000	Length (m)	59.9	Max Percolation (l/s)	199.7	Slope (1:X)	50.0	Safety Factor	2.0	Depression Storage (mm)	5	Porosity	0.30	Evaporation (mm/day)	3	Invert Level (m)	24.050	Membrane Depth (mm)	0	Infiltration Coefficient Base (m/hr)	0.00000	Width (m)	15.8	Membrane Percolation (mm/hr)	1000	Length (m)	29.0	Max Percolation (l/s)	127.3	Slope (1:X)	80.0	Safety Factor	2.0	Depression Storage (mm)	5	Porosity	0.30	Evaporation (mm/day)	3	Invert Level (m)	21.900	Membrane Depth (mm)	0	Infiltration Coefficient Base (m/hr)	0.00000	Width (m)	12.8	Membrane Percolation (mm/hr)	1000	Length (m)	44.5	Max Percolation (l/s)	158.2	Slope (1:X)	50.0	Safety Factor	2.0	Depression Storage (mm)	5	Porosity	0.30	Evaporation (mm/day)	3	Invert Level (m)	21.100	Membrane Depth (mm)	0	Infiltration Coefficient Base (m/hr)	0.00000	Max Percolation (l/s)	81.8	Membrane Percolation (mm/hr)	1000	Safety Factor	2.0
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Innovyze Network 2020.1																										
<u>Porous Car Park Manhole: S47, DS/PN: S12.000</u> <table><tr><td>Porosity</td><td>0.30</td><td>Slope (1:X)</td><td>50.0</td></tr><tr><td>Invert Level (m)</td><td>21.100</td><td>Depression Storage (mm)</td><td>5</td></tr><tr><td>Width (m)</td><td>26.3</td><td>Evaporation (mm/day)</td><td>3</td></tr><tr><td>Length (m)</td><td>11.2</td><td>Membrane Depth (mm)</td><td>0</td></tr></table> <u>Tank or Pond Manhole: S25, DS/PN: S1.024</u> Invert Level (m) 19.200 <table><tr><th>Depth (m)</th><th>Area (m²)</th><th>Depth (m)</th><th>Area (m²)</th></tr><tr><td>0.000</td><td>1788.0</td><td>0.900</td><td>3216.0</td></tr></table>			Porosity	0.30	Slope (1:X)	50.0	Invert Level (m)	21.100	Depression Storage (mm)	5	Width (m)	26.3	Evaporation (mm/day)	3	Length (m)	11.2	Membrane Depth (mm)	0	Depth (m)	Area (m²)	Depth (m)	Area (m²)	0.000	1788.0	0.900	3216.0
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Depth (m)	Area (m²)	Depth (m)	Area (m²)																							
0.000	1788.0	0.900	3216.0																							
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APPENDIX H

Drawings Extracted from the Site-Wide Flood Risk Assessment:

(SuDS Management Train, Site-Wide Preliminary Water Management Strategy Plan (sheet 2 of 2), Typical Layout of Strategic SuDS Features to Inform Detailed Design)

KEY



Rainfall



Interception Losses (Evapotranspiration / Infiltration)



Water Reuse



Water Quality Device (Oil Separator / Vortex Separator)



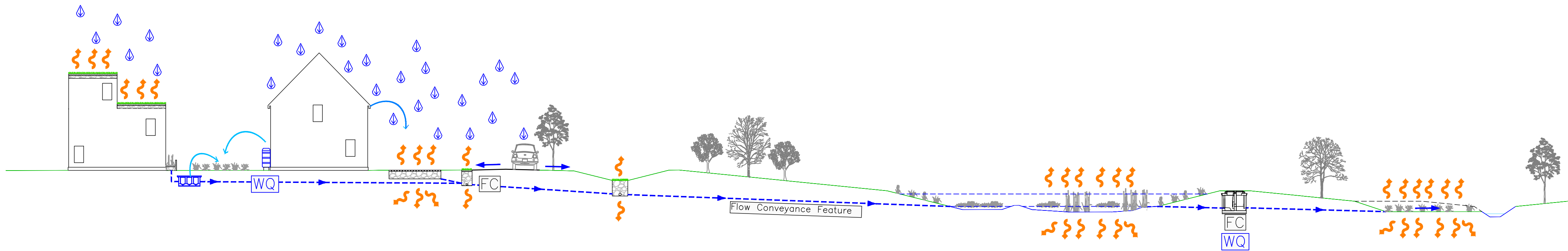
Flow Control Device (Orifice / Vortex Flow Control)



Overland Flow



Surface Water Sewer (connecting SuDS Features)



DEVELOPMENT TYPE	Commercial / Public	Residential	Private / Shared Parking	Public Highway	Conveyance Features		Green Infrastructure / Public Open Space	Watercourse Corridor
MANAGEMENT TRAIN	PREVENTION SOURCE CONTROL SITE / REGIONAL CONTROL							
SuDS FEATURE	Rainwater Harvesting (Reuse for irrigation e.g Rainwater Butts) Green Roofs Bioretention Areas / Rain Gardens Rills	Rainwater Harvesting (Reuse for irrigation e.g Rainwater Butts) Porous Paving Tanks Flow Control Devices		Swales Bioretention Areas Filter Strips Filter Drains			Strategic SuDS incorporating sediment forbays, wetland areas, micropools at the outlet Flow Control Devices Water Quality Devices	Outfall Swale
MAINTENANCE RESPONSIBILITY	Commercial or Public Tenants	Homeowners	Homeowners / Management Company	Highway Authority	Southern Water / Management Company		Southern Water / Management Company	Southern Water / Management Company



Stratton Park House, Wanborough Road
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For Planning
This drawing is produced for the purposes of supporting a planning application and should not be relied upon for tender, pricing, or construction purposes.

NOTES

- Drawing to be read in conjunction with Flood Risk Assessment and Water Management Strategy produced by PFA Consulting (Document Ref: G277-DOC01 Dated November 2020) and Preliminary Water Management Strategy Plan (Drawing No. G277/21 & 22).
- Future design of upstream Source Control & Site Control to take account of the principles established in the Water Management Strategy and this drawing.

Rev	Date	Description	Drawn	Check
#	26.11.20	First Issue.	BF	MWS

Status

FOR PLANNING

Client

Eastleigh Borough Council

Project

One Horton Heath
Hampshire

Drawing Title

SuDS Management Train

Drawing No.

G277/24

Date: November 2020

Scale: NTS @ A1

E-Mail: bfox@pfaplc.com

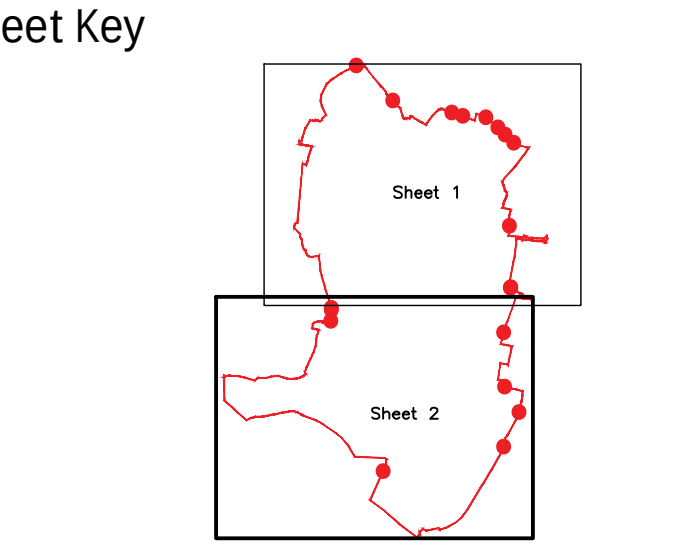


Strategic SuDS Features Schedule							
Strategic SuDS Feature	Contributing Catchment (ha)	% of impermeability	Q ₁₀₀ for Catchment (l/s)	1 in 1 Year Storage Volume (m³)	1 in 1 Year Resulting Discharge (l/s)	1 in 100+40%CC Storage Volume (m³)	1 in 100+40%CC Resulting Discharge (l/s)
A	2.037	65	18.4	165	17.8	802	18.3
	0.260	100					
B	3.750	65	30.0	260	28.1	1224	29.9
C	6.808	65	54.5	405	50.1	2172	54.4
D	3.381	65	27.1	244	23.7	1112	27
E	3.650	65	32.3	337	24.8	1706	32.3
	0.382	100					
F	3.360	65	26.9	241	23.9	1103	26.9
G	2.762	65	41.5	267	34.7	1450	40.3
	2.415	50					
H	1.603	30	70.8	683	44.1	2694	70.8
	0.409	100					
I	6.831	65	29.6	314	26.7	1339	28.7
	1.556	65					
J	2.139	65	13.6	127	11.0	560	13.5
K	1.696	65	39.6	433	32.5	1860	39.5
	3.894	65					
L	1.046	100	17.7	156	16.2	728	17.7
M	2.213	65	46.1	477	31.1	1924	46.0
N	5.759	65	16.3	150	15.5	878	16.3
EM(Employment)	2.029	85					

For Planning
This drawing is produced for the purposes of supporting a planning application and should not be relied upon for tender, pricing, or construction purposes.

- NOTES**
- This drawing should be read in conjunction with PFA Consulting's Flood Risk Assessment (Doc ref: G277-DOCD1 FRA - Issue 1) dated Dec 2020.
 - This drawing is based on the Topographical Survey undertaken by Mayer Brown Limited, Drawing No:IR.SAV.1.21.1, Rev E, dated 05 May 2014.
 - This site layout is based on Thrive's Concept Masterplan, Drawing number EAST200403 CMP 01 Rev P13, dated 04.12.20
 - Main River designation based on Flood Map for Planning.
 - All SuDS features shown on this drawing are subject to detailed design.
 - Typical Layout of Strategic SuDS Features to Inform Detailed Design shown on drawing number G277/23.
 - Detailed design of highway drainage features to provide attenuation and restrict flow to the downstream watercourse system. Sufficient attenuation should be provided to ensure discharge rates are restricted for all return periods up to the 100 year design event, including an appropriate allowance for climate change. Refer to PFA's Flood Risk Assessment (dated Dec 2020) document for guidance on flow rate restrictions.

- Key**
- Risk of Flooding from Surface Water Outfall
 - Basin Catchment Allocation
 - Flood Zone 2
 - Flood Zone 3
 - Indicative Location of Foul Water Sewage Pumping Station with 10m clear offset
 - 5m offset from Watercourse
 - 5m offset from Main Rivers
 - Opportunity for scale conveyance feature
 - Direction of Excessance Flows
 - Outlet from SuDS Feature to be conveyance swale where feasible
 - Indicative Proposed Foul Rising Main
 - Indicative Proposed Strategic Foul Sewer
 - Indicative Foul flow direction



Rev	Date	Description	Drawn	Check
#	27/11/20	First Issue	DAB	WVG
A	11/12/20	Revised to latest Masterplan & link road design	DAB	SAM

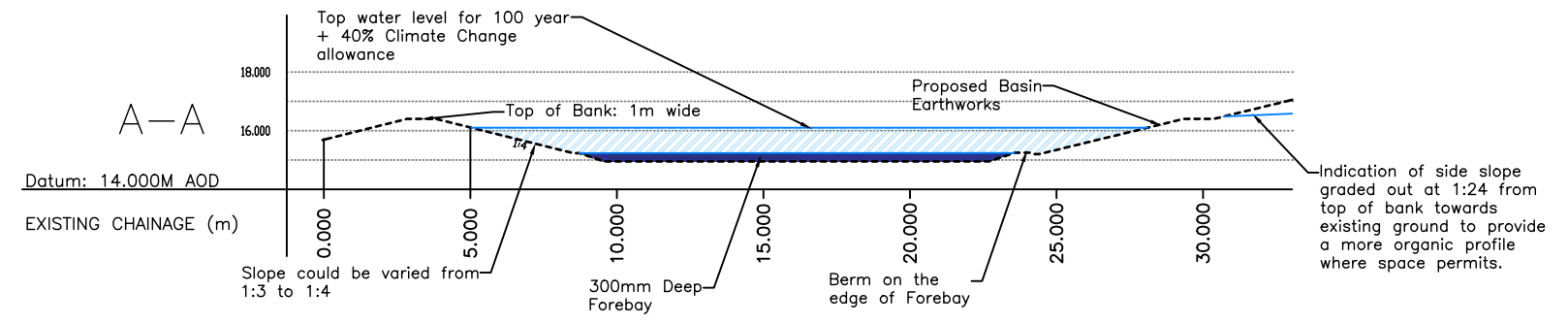
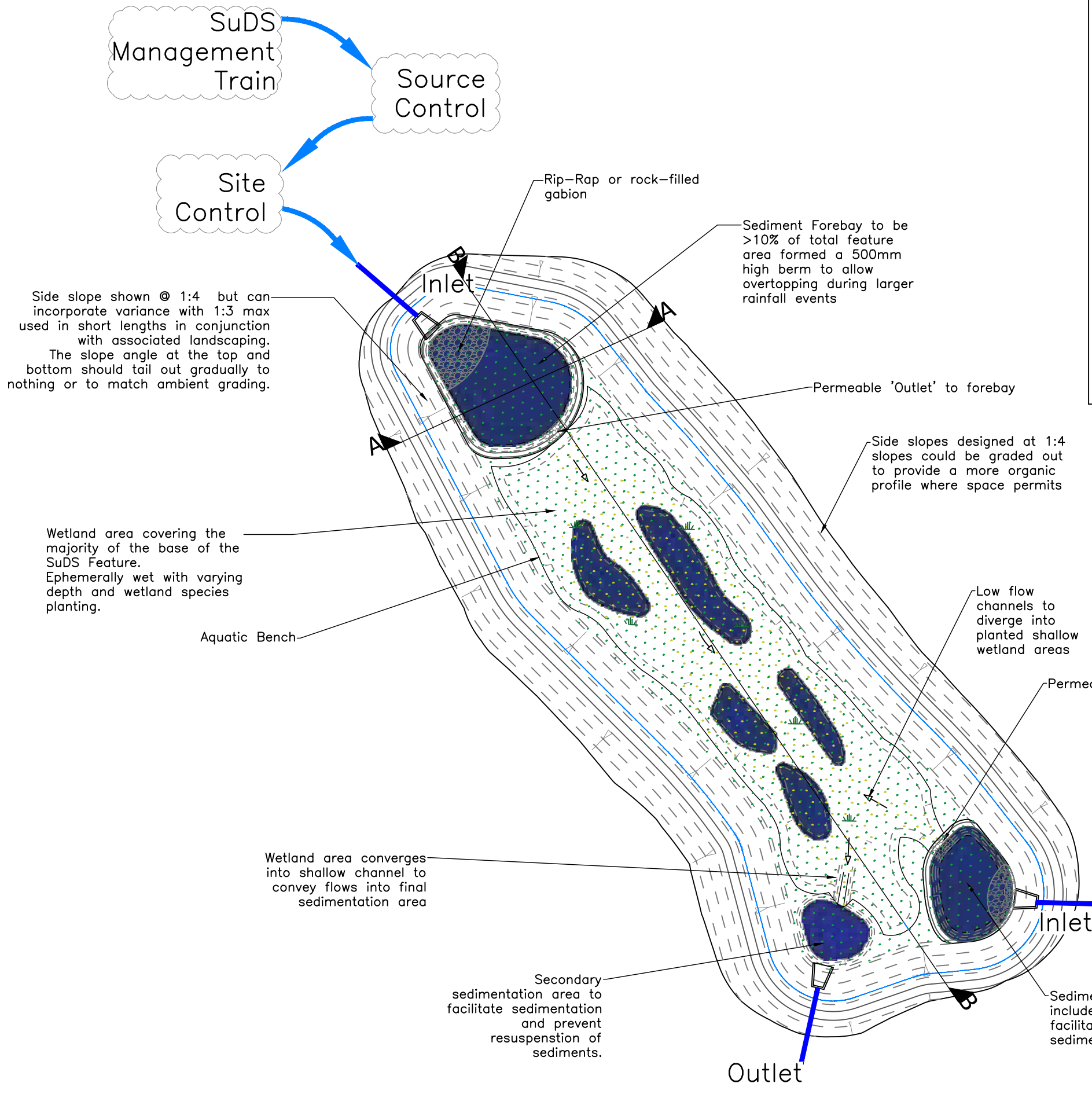
Status
FOR PLANNING

Client
Eastleigh Borough Council

Project
**One Horton Heath
Hampshire**

Drawing Title
**Preliminary Water
Management Strategy Plan
(Sheet 2 of 2)**

Drawing No. **G277/22** Rev A
Date: November 2020 Scale: 1:1250 @ A0
E-Mail: DBuciak@pfapl.com



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For Planning
This drawing is produced for the purposes of supporting a planning application and should not be relied upon for tender, pricing, or construction purposes.

NOTES
1. This drawing shows the typical SuDS feature arrangements for the SuDS features shown on the Preliminary Water Management Strategy Plan, Drawing number G277/21 & G277/22.
2. Upstream SuDS management train principles are outlined in the PFA's Water Management Strategy and depicted on drawing G277/24 dated November 2020.

Rev	Date	Description	Drawn	Check
#	26/11/20	First Issue.	DAB	SAM

Status
FOR PLANNING

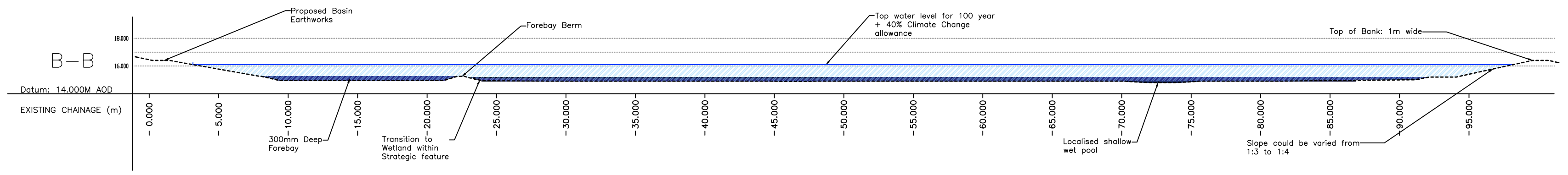
Client
Eastleigh Borough Council

Project
One Horton Heath Hampshire

Drawing Title
Typical Layout of Strategic SuDS Features to Inform Detailed Design

Drawing No.
G277/23

Date: November 2020 Scale: As shown
E-Mail: DBuciak@pfapl.com



APPENDIX I

Maintenance Schedules

TABLE 17.1 Operation and maintenance requirements for swales

Maintenance schedule	Required action	Typical frequency
Regular maintenance	Remove litter and debris	Monthly, or as required
	Cut grass – to retain grass height within specified design range	Monthly (during growing season), or as required
	Manage other vegetation and remove nuisance plants	Monthly at start, then as required
	Inspect inlets, outlets and overflows for blockages, and clear if required	Monthly
	Inspect infiltration surfaces for ponding, compaction, silt accumulation, record areas where water is ponding for > 48 hours	Monthly, or when required
	Inspect vegetation coverage	Monthly for 6 months, quarterly for 2 years, then half yearly
	Inspect inlets and facility surface for silt accumulation, establish appropriate silt removal frequencies	Half yearly
Occasional maintenance	Reseed areas of poor vegetation growth, alter plant types to better suit conditions, if required	As required or if bare soil is exposed over 10% or more of the swale treatment area
Remedial actions	Repair erosion or other damage by re-turfing or reseeding	As required
	Relevel uneven surfaces and reinstate design levels	As required
	Scarify and spike topsoil layer to improve infiltration performance, break up silt deposits and prevent compaction of the soil surface	As required
	Remove build-up of sediment on upstream gravel trench, flow spreader or at top of filter strip	As required
	Remove and dispose of oils or petrol residues using safe standard practices	As required

TABLE 20.15 Operation and maintenance requirements for pervious pavements

Maintenance schedule	Required action	Typical frequency
Regular maintenance	Brushing and vacuuming (standard cosmetic sweep over whole surface)	Once a year, after autumn leaf fall, or reduced frequency as required, based on site-specific observations of clogging or manufacturer's recommendations – pay particular attention to areas where water runs onto pervious surface from adjacent impermeable areas as this area is most likely to collect the most sediment
Occasional maintenance	Stabilise and mow contributing and adjacent areas	As required
	Removal of weeds or management using glyphosate applied directly into the weeds by an applicator rather than spraying	As required – once per year on less frequently used pavements
Remedial Actions	Remediate any landscaping which, through vegetation maintenance or soil slip, has been raised to within 50 mm of the level of the paving	As required
	Remedial work to any depressions, rutting and cracked or broken blocks considered detrimental to the structural performance or a hazard to users, and replace lost jointing material	As required
	Rehabilitation of surface and upper substructure by remedial sweeping	Every 10 to 15 years or as required (if infiltration performance is reduced due to significant clogging)
Monitoring	Initial inspection	Monthly for three months after installation
	Inspect for evidence of poor operation and/or weed growth – if required, take remedial action	Three-monthly, 48 h after large storms in first six months
	Inspect silt accumulation rates and establish appropriate brushing frequencies	Annually
	Monitor inspection chambers	Annually

Many of the specific maintenance activities for pervious pavements can be undertaken as part of a general site cleaning contract (many car parks or roads are swept to remove litter and for visual reasons to keep them tidy) and therefore, if litter management is already required at site, this should have marginal cost implications.

Generally, pervious pavements require less frequent gritting in winter to prevent ice formation. There is also less risk of ice formation after snow melt, as the melt water drains directly into the underlying sub-base and does not have chance to refreeze. A slight frost may occur more frequently on the surface of pervious pavements compared to adjacent impermeable surfaces, but this is only likely to last for a few hours. It does not happen in all installations and, if necessary, this can be dealt with by application of salt. It is not likely to pose a hazard to vehicle movements.

► Generic health and safety guidance is presented in **Chapter 36**.

CDM 2015 requires designers to ensure that all maintenance risks have been identified, eliminated, reduced and/or controlled where appropriate. This information will be required as part of the health and safety file.

TABLE 22.1 Operation and maintenance requirements for detention basins

Maintenance schedule	Required action	Typical frequency
Regular maintenance	Remove litter and debris	Monthly
	Cut grass – for spillways and access routes	Monthly (during growing season), or as required
	Cut grass – meadow grass in and around basin	Half yearly (spring – before nesting season, and autumn)
	Manage other vegetation and remove nuisance plants	Monthly (at start, then as required)
	Inspect inlets, outlets and overflows for blockages, and clear if required.	Monthly
	Inspect banksides, structures, pipework etc for evidence of physical damage	Monthly
	Inspect inlets and facility surface for silt accumulation. Establish appropriate silt removal frequencies.	Monthly (for first year), then annually or as required
	Check any penstocks and other mechanical devices	Annually
	Tidy all dead growth before start of growing season	Annually
	Remove sediment from inlets, outlet and forebay	Annually (or as required)
	Manage wetland plants in outlet pool – where provided	Annually (as set out in Chapter 23)
Occasional maintenance	Reseed areas of poor vegetation growth	As required
	Prune and trim any trees and remove cuttings	Every 2 years, or as required
	Remove sediment from inlets, outlets, forebay and main basin when required	Every 5 years, or as required (likely to be minimal requirements where effective upstream source control is provided)
Remedial actions	Repair erosion or other damage by reseedling or re-turfing	As required
	Realignment of rip-rap	As required
	Repair/rehabilitation of inlets, outlets and overflows	As required
	Relevel uneven surfaces and reinstate design levels	As required

22.13 REFERENCE

KENNARD, M F, HOSKINS, C G and FLETCHER, M (1996) *Small embankment reservoirs*, R161, CIRIA, London, UK (ISBN: 978-0-86017-461-5). Go to: www.ciria.org

Statutes

Reservoir Act 1975 (c.23)

Health and Safety at Work (etc) Act 1974 (c.37)

Building Act 1984 (c.55)

Flood and Water Management Act 2010 (c.29)

Construction (Design and Management) Regulations (CDM) 2015

TABLE 14.2 An example of operation and maintenance requirements for a proprietary treatment system

Maintenance schedule	Required action	Typical frequency
Routine maintenance	Remove litter and debris and inspect for sediment, oil and grease accumulation	Six monthly
	Change the filter media	As recommended by manufacturer
	Remove sediment, oil, grease and floatables	As necessary – indicated by system inspections or immediately following significant spill
Remedial actions	Replace malfunctioning parts or structures	As required
Monitoring	Inspect for evidence of poor operation	Six monthly
	Inspect filter media and establish appropriate replacement frequencies	Six monthly
	Inspect sediment accumulation rates and establish appropriate removal frequencies	Monthly during first half year of operation, then every six months

should be undertaken whenever access for maintenance is required. Removal of oil, silt and other pollutants must be in accordance with the appropriate waste management legislation.

Maintenance responsibility for all systems should be placed with an appropriate organisation, and Maintenance Plans and schedules should be developed during the design phase.

- Further detail on the preparation of maintenance specifications and schedules of work is given in **Chapter 32**.

Table 14.2 provides guidance on the type of operation and maintenance schedule that may be appropriate for a proprietary treatment system. The list of actions is not exhaustive and some actions may not always be required.

CDM 2015 requires designers to ensure that all maintenance risks have been identified, eliminated, reduced and/or controlled where appropriate. This information will be required as part of the health and safety file.

- Generic health and safety guidance is presented in **Chapter 36**.

14.12.2 Oil water separators

Specific requirements for oil/water separators are provided in PPG3 (EA/SEPA/EHSNI, 2006). The following items should be undertaken every six months as a minimum:

- check volume of sludge
- check thickness of light liquid
- check function of automatic closure device
- empty the separator, if required
- check the coalescing material and clean or change if necessary (class 1 only)
- check the function of the warning device (if fitted)

General inspection of the integrity of oil/water separators should occur at a maximum frequency of five years, and should cover the following:

- watertightness of system

Project Name : Horton Heath, Eastleigh

Job No : 19-367

Note Title : Flood Risk Assessment and Drainage Strategy - Addendum 1

Author : JW

Checked : JH

Approved : GG

Date : May 2021

1.0 INTRODUCTION

1.1 *Background*

1.1.1 Odyssey was commissioned by Eastleigh Borough Council (EBC) to undertake a Flood Risk Assessment (FRA) and Water Management Strategy to be submitted in association with a planning application in support of a proposed development for a site at Horton Heath, Eastleigh.

1.1.2 The wider One Horton Heath development has obtained planning permission via two separate applications. The new composite outline planning permission for the wider site would increase the number of proposed dwellings from circa 1,500 to 2,500. The FRA and Water Management Strategy relates to the First Residential Parcel (FRP) of the development for 393 of the 2,500 dwellings.

1.1.3 The FRA and Water Management Strategy were submitted to EBC as part of the full planning application (reference F/20/89500) for the proposed FRP development.

1.1.4 Following the submission of the application, comments were received from Hampshire County Council (HCC) in their capacity as the Lead Local Flood Authority (LLFA), in February 2021 (HCC reference: SWM/2021/0019). A copy of the LLFA consultation comments is provided in **Appendix A**.

1.1.5 This FRA Addendum, which should be read in conjunction with the submitted FRA and Water Management Strategy (reference 19-367-01B), provides additional information as requested by the LLFA.

2.0 RESPONSE TO HAMPSHIRE COUNTY COUNCIL COMMENTS

2.1.1 HCC provided the following comments (*in bold italics*) in February 2021. Odyssey's response directly follows each comment.

“The drainage plan shows a series of outfalls into the existing ditch. However, the fall of the ditch is erratic with some sections backfalling and there being a relatively shallow depth throughout. No levels have been provided at the headwalls so it is not clear what the final outfall levels will be nor is there any information about the typical water level within the ditch. This should be provided to ensure that the outfalls will be functional and water will flow appropriately within the ditch.”

2.1.2 It is proposed to discharge Basin I, Basin K and Basin L directly into the existing ditch.

2.1.3 The pipework from Basin I has been designed to outfall at an approximate invert level of 19.88 metres above ordnance datum (m AOD). The bed level of the ditch in the location of the proposed outfall is 19.58. The proposed pipe would outfall at 300mm above the invert level of the ditch.

2.1.4 The pipework from Basin K has been designed to outfall at an approximate invert level of 18.05m AOD. The bed level of the ditch in the location of the proposed outfall is 17.90m AOD. The proposed pipe would outfall 150mm above the invert of the ditch.

2.1.5 The pipework from Basin L has been designed to outfall at an approximate invert level of 17.00m AOD. The bed level of the ditch in the location of the proposed outfall is 16.60m AOD. The proposed pipe would outfall at 400mm above the invert of the ditch.

2.1.6 The typical water level within the ditch is currently unknown.

“If water levels in the ditch rise, it is not clear what the impact will be on the drainage system so if modelling is not available, the drainage should be modelled using a surcharged outfall to understand the potential flood risk.”

2.1.7 The drainage networks have been modelled using a submerged outfall for the 1 in 100 year storm with 40% allowance for climate change. The results are presented in **Appendix B**. The results show that even with a submerged outfall, none of the proposed drainage networks would flood.

“Permeable paving is proposed within the parking areas. Please confirm whether these are lined given that parts of the site could suffer with high groundwater.”

2.1.8 All proposed permeable paving would be lined.

“A series of what we believe to be swales are identified adjacent to the watercourse and in various locations around the development. These do not appear to be connected and there is a section that appears to go through basin I. There are also sections that appear to clash with the proposed access roads between the dwellings and the ponds. Please clarify these elements particularly as there appears to be an existing ditch in this location which would require ordinary watercourse consent if any alterations are proposed.”

2.1.9 The proposed swales are shown in blue on the drainage strategy. The basin and access roads are shown over existing hedges, which would be removed. The updated drawing is presented in **Appendix C**.

2.1.10 The proposed swales to the south-west of the site would drain into the main drainage system. The two enhanced swales on the north side of the site would be incorporated into the pipework; directing water through the enhanced swales would provide additional treatment. The outfall into Basin K has also been moved to better accommodate the swales. The storage volume that would be provided by the proposed swales has not been included in the hydraulic model as a conservative approach.

“Detailed hydraulic calculations have been provided but these do not appear to include the results summary pages. Please can this be provided.”

2.1.11 Updated hydraulic calculations including the results summary pages are presented in **Appendix B**.

“It is stated that a maintenance company will be responsible for the drainage assets. Please note that our response is separate from Highways and if the roads are to be adopted, maintenance responsibilities will need to be discussed with the Highways team.”

2.1.12 All proposed adoptable highway drains and adoptable surface water sewers would be maintained by HCC Highways and Southern Water respectively. The remaining private drainage would be maintained by the appointed maintenance company.

“Not with standing the above comments, we would request a condition to ensure clarity on who will be responsible for the drainage maintenance responsibility with a clear plan detailing this as well as confirmation that the relevant bodies will adopt the relevant features”

2.1.13 A condition of this nature would be accepted.

APPENDIX A

LLFA Comments



Hampshire County Council

*Economy, Transport and Environment Department
Elizabeth II Court West, The Castle
Winchester, Hampshire SO23 8UD*

Tel: 0300 555 1375 (General Enquiries)
0300 555 1388 (Roads and Transport)
0300 555 1389 (Recycling Waste & Planning)
Textphone 0300 555 1390
Fax 01962 847055
www.hants.gov.uk

<i>Enquiries to</i>	Sarah Reghif	<i>My reference</i>	SWM/2021/0019
<i>Direct Line</i>	0370 779 7497	<i>Your reference</i>	F/20/89500
<i>Date</i>	5 February 2021	<i>Email</i>	SWM.consultee@hants.gov.uk

Dear Sir/Madam,

Erection of 393 residential units with public open space, hard and soft landscape works, internal roads, footway/cycleways, associated parking spaces, substation, SuDS and associated infrastructure (This application is subject to Environmental Impact Assessment). at Land west of Burnetts Lane, Eastleigh, SO30 2HH

Hampshire County Council as Lead Local Flood Authority has provided comments in relation to the above application in our role as statutory consultee on surface water drainage for major developments.

In order to assist applicants in providing the correct information to their Local Planning Authority for planning permission, Hampshire County Council has set out the information it requires to provide a substantive response at <https://www.hants.gov.uk/landplanningandenvironment/environment/flooding/planning>

The County Council has reviewed the following documents relating to the above application:

- Flood Risk Assessment and Water Management Strategy rev B
- PFA Flood Risk Assessment Issue 1 dated December 2020
- Phase 1 – Ground Investigation Desk Study Assessment ref S2 P01
- Phase 2 – Ground Investigation Report ref: S2 P1

In order for us to provide a substantive response, the following information is required:

The drainage plan shows a series of outfalls into the existing ditch. However, the fall of the ditch is erratic with some sections backfalling and there being a relatively shallow depth throughout. No levels have been provided at the

*Director of Economy, Transport and Environment
Stuart Jarvis BSc DipTP FCIHT MRTPI*

headwalls so it is not clear what the final outfall levels will be nor is there any information about the typical water level within the ditch. This should be provided to ensure that the outfalls will be functional and water will flow appropriately within the ditch.

If water levels in the ditch rise, it is not clear what the impact will be on the drainage system so if modelling is not available, the drainage should be modelled using a surcharged outfall to understand the potential flood risk.

Permeable paving is proposed within the parking areas. Please confirm whether these are lined given that parts of the site could suffer with high groundwater.

A series of what we believe to be swales are identified adjacent to the watercourse and in various locations around the development. These do not appear to be connected and there is a section that appears to go through basin I. There are also sections that appear to clash with the proposed access roads between the dwellings and the ponds. Please clarify these elements particularly as there appears to be an existing ditch in this location which would require ordinary watercourse consent if any alterations are proposed.

Detailed hydraulic calculations have been provided but these do not appear to include the results summary pages. Please can this be provided.

It is stated that a maintenance company will be responsible for the drainage assets. Please note that our response is separate from Highways and if the roads are to be adopted, maintenance responsibilities will need to be discussed with the Highways team.

Notwithstanding the above points, we would request a condition to ensure clarity on who will be responsible for the drainage maintenance responsibilities with a clear plan detailing this as well as confirmation that the relevant bodies will adopt the relevant features.

As a statutory consultee, the County Council has a duty to respond to consultations within **21 days**. The 21 day period will not begin until we have received sufficient information to enable us to provide a meaningful response.

Please ensure all data is sent to us via the relevant Local Planning Authority.

For guidance on providing the correct information, we recommend you use our **Surface Water Management Pre-application service** which provides clear guidance on what is required for us to recommend that planning permission is granted and consider the works as best practise. For full details, please visit: <https://www.hants.gov.uk/landplanningandenvironment/environment/flooding/planning> and click on [pre-application advice request form](#).

This response has been provided using the best knowledge and information submitted as part of the planning application at the time of responding and is reliant on the accuracy of that information.

Yours faithfully,



Flood and Water Management Team
Economy, Transport & Environment Department,
Hampshire County Council, 1st Floor, Ell Court West,
The Castle, Winchester, Hampshire SO23 8UD

Web:

<https://www.hants.gov.uk/landplanningandenvironment/environment/flooding>

General guidance for the application

It is important to ensure that the long-term maintenance and responsibility for Sustainable Drainage Systems is agreed between the Local Planning Authority and the applicant before planning permission is granted. This should involve discussions with those adopting and/or maintaining the proposed systems, which could include the Highway Authority, Planning Authority, Parish Councils, Water Companies and private management companies.

For SuDS systems to be adopted by Hampshire Highways it is recommended that you visit the website at:

<https://www.hants.gov.uk/transport/developers/constructionstandards> for guidance on which drainage features would be suitable for adoption.

Where the proposals are connecting to an existing drainage system it is likely that the authorities responsible for maintaining those systems will have their own design requirements. These requirements will need to be reviewed and agreed as part of any surface water drainage scheme.

Works in relation to ordinary watercourses

PLEASE NOTE: *If the proposals include works to an ordinary watercourse, under the Land Drainage Act 1991, as amended by the Flood and Water Management Act 2010, prior consent from the Lead Local Flood Authority is required. **This consent is required as a separate permission to planning.***

Information on ordinary watercourse consenting can be found at the following link

<https://www.hants.gov.uk/landplanningandenvironment/environment/flooding/c/hangewatercourse>

It is strongly recommended that this information is reviewed before Land Drainage consent application is made.

*For guidance on providing the correct information, we recommend you use our **Ordinary Watercourse Consents Pre-application service** and help avoid delays occurring at the formal application stage. A Pre-application service for Ordinary Watercourse Consents is available, allowing consents to go through in a smoother, often more timely manner. For full information please visit:*

<https://www.hants.gov.uk/landplanningandenvironment/environment/flooding/c/hangewatercourse>

APPENDIX B

MicroDrainage Calculations

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STORM SEWER DESIGN by the Modified Rational Method

Design Criteria for Storm

Pipe Sizes STANDARD Manhole Sizes STANDARD

FSR Rainfall Model - England and Wales

Return Period (years)	100	PIMP (%)	100
M5-60 (mm)	19.900	Add Flow / Climate Change (%)	0
Ratio R	0.350	Minimum Backdrop Height (m)	0.200
Maximum Rainfall (mm/hr)	50	Maximum Backdrop Height (m)	1.500
Maximum Time of Concentration (mins)	30	Min Design Depth for Optimisation (m)	1.200
Foul Sewage (l/s/ha)	0.000	Min Vel for Auto Design only (m/s)	1.00
Volumetric Runoff Coeff.	0.750	Min Slope for Optimisation (1:X)	500

Designed with Level Soffits

Time Area Diagram for Storm

Time (mins)	Area (ha)	Time (mins)	Area (ha)
0-4	0.386	4-8	0.602

Total Area Contributing (ha) = 0.988

Total Pipe Volume (m³) = 41.916

Network Design Table for Storm

« - Indicates pipe capacity < flow

PN	Length (m)	Fall (m)	Slope (1:X)	I.Area (ha)	T.E. (mins)	Base Flow (l/s)	k (mm)	HYD SECT	DIA (mm)	Section Type	Auto Design
S1.000	4.862	0.161	30.2	0.023	4.00	0.0	0.600	o	150	Pipe/Conduit	
S1.001	33.610	0.862	39.0	0.071	0.00	0.0	0.600	o	225	Pipe/Conduit	
S1.002	23.881	0.353	67.7	0.036	0.00	0.0	0.600	o	225	Pipe/Conduit	
S1.003	9.044	0.270	33.5	0.050	0.00	0.0	0.600	o	225	Pipe/Conduit	
S1.004	33.270	0.746	44.6	0.005	0.00	0.0	0.600	o	225	Pipe/Conduit	

Network Results Table

PN	Rain (mm/hr)	T.C. (mins)	US/IL (m)	Σ I.Area (ha)	Σ Base Flow (l/s)	Foul (l/s)	Add Flow (l/s)	Vel (m/s)	Cap (l/s)	Flow (l/s)
S1.000	50.00	4.04	26.241	0.023	0.0	0.0	0.0	1.84	32.5	3.1
S1.001	50.00	4.31	26.005	0.094	0.0	0.0	0.0	2.10	83.6	12.7
S1.002	50.00	4.56	25.143	0.130	0.0	0.0	0.0	1.59	63.3	17.6
S1.003	50.00	4.63	24.790	0.180	0.0	0.0	0.0	2.27	90.2	24.4
S1.004	50.00	4.91	24.520	0.185	0.0	0.0	0.0	1.96	78.1	25.1

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Network Design Table for Storm

PN	Length (m)	Fall (m)	Slope (1:X)	I.Area (ha)	T.E. (mins)	Base Flow (l/s)	k (mm)	HYD SECT	DIA (mm)	Section Type	Auto Design
S4.002	15.165	0.283	53.6	0.011	0.00	0.0	0.600	o	225	Pipe/Conduit	
S1.012	7.910	0.025	313.0	0.030	0.00	0.0	0.600	o	375	Pipe/Conduit	
S1.013	6.861	0.020	350.0	0.000	0.00	0.0	0.600	o	450	Pipe/Conduit	
S1.014	6.475	0.097	66.8	0.007	0.00	0.0	0.600	o	450	Pipe/Conduit	
S1.015	17.327	0.050	350.0	0.004	0.00	0.0	0.600	o	450	Pipe/Conduit	
S1.016	36.838	0.126	292.4	0.000	0.00	0.0	0.600	o	450	Pipe/Conduit	
S6.000	20.356	0.136	150.0	0.011	4.00	0.0	0.600	o	150	Pipe/Conduit	
S6.001	28.557	0.190	150.0	0.011	0.00	0.0	0.600	o	150	Pipe/Conduit	
S6.002	56.866	1.400	40.6	0.019	0.00	0.0	0.600	o	150	Pipe/Conduit	
S6.003	27.554	0.580	47.5	0.050	0.00	0.0	0.600	o	150	Pipe/Conduit	
S6.004	27.972	0.186	150.4	0.000	0.00	0.0	0.600	o	150	Pipe/Conduit	
S6.005	35.180	0.514	68.5	0.000	0.00	0.0	0.600	o	150	Pipe/Conduit	
S1.017	48.497	0.550	88.2	0.000	0.00	0.0	0.600	o	450	Pipe/Conduit	
S1.018	13.823	0.168	82.3	0.000	0.00	0.0	0.600	o	300	Pipe/Conduit	

Network Results Table

PN	Rain (mm/hr)	T.C. (mins)	US/IL (m)	Σ I.Area (ha)	Σ Base Flow (l/s)	Foul (l/s)	Add Flow (l/s)	Vel (m/s)	Cap (l/s)	Flow (l/s)
S4.002	50.00	4.43	21.425	0.152	0.0	0.0	0.0	1.79	71.2	20.6
S1.012	50.00	5.88	20.992	0.886	0.0	0.0	0.0	1.02	112.5	120.0
S1.013	50.00	5.99	20.892	0.886	0.0	0.0	0.0	1.08	171.9	120.0
S1.014	50.00	6.03	20.872	0.893	0.0	0.0	0.0	2.49	396.2	120.9
S1.015	50.00	6.30	20.775	0.897	0.0	0.0	0.0	1.08	171.9	121.5
S1.016	50.00	6.82	20.726	0.897	0.0	0.0	0.0	1.18	188.3	121.5
S6.000	50.00	4.41	23.906	0.011	0.0	0.0	0.0	0.82	14.5	1.5
S6.001	50.00	5.00	23.770	0.022	0.0	0.0	0.0	0.82	14.5	3.0
S6.002	50.00	5.59	23.580	0.041	0.0	0.0	0.0	1.58	28.0	5.6
S6.003	50.00	5.91	22.180	0.091	0.0	0.0	0.0	1.46	25.9	12.3
S6.004	50.00	6.48	21.600	0.091	0.0	0.0	0.0	0.82	14.4	12.3
S6.005	50.00	6.96	21.414	0.091	0.0	0.0	0.0	1.22	21.5	12.3
S1.017	50.00	7.33	20.600	0.988	0.0	0.0	0.0	2.17	344.5	133.8
S1.018	50.00	7.47	20.050	0.988	0.0	0.0	0.0	1.73	122.6	133.8

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Manhole Schedules for Storm									
MH Name	MH CL (m)	MH Depth (m)	MH Connection	MH Diam., L*W (mm)	PN	Pipe Out Invert Level (m)	Pipe Diameter (mm)	PN	Pipe Invert Level (m)
SPP1	27.241	1.000	Open Manhole	1200	S1.000	26.241	150		
SHBRAKE PP1	27.430	1.425	Open Manhole	1200	S1.001	26.005	225	S1.000	26.005
S3	26.568	1.425	Open Manhole	1200	S1.002	25.143	225	S1.001	25.143
S4	26.215	1.425	Open Manhole	1200	S1.003	24.790	225	S1.002	24.790
S5	25.945	1.425	Open Manhole	1200	S1.004	24.520	225	S1.003	24.520
SPP2	25.217	1.000	Open Manhole	1200	S2.000	24.217	150		
SHBRAKE PP2	25.800	1.713	Open Manhole	1200	S2.001	24.087	225	S2.000	24.087
S8	25.458	1.528	Open Manhole	1200	S2.002	23.930	225	S2.001	23.930
S9	25.390	1.489	Open Manhole	1200	S2.003	23.901	225	S2.002	23.901
S10	25.202	1.503	Open Manhole	1200	S1.005	23.699	300	S1.004	23.699
								S2.003	23.901
S11	24.804	1.503	Open Manhole	1200	S1.006	23.301	300	S1.005	23.301
S12	24.441	1.504	Open Manhole	1200	S1.007	22.938	300	S1.006	22.938
S13	24.097	1.503	Open Manhole	1200	S1.008	22.594	300	S1.007	22.594
S14	24.058	1.425	Open Manhole	1200	S3.000	22.633	225		
S15	23.758	1.425	Open Manhole	1200	S3.001	22.333	225	S3.000	22.333
S16	23.662	1.541	Open Manhole	1200	S1.009	22.121	300	S1.008	22.121
								S3.001	22.333
S17	23.127	1.503	Open Manhole	1200	S1.010	21.625	300	S1.009	21.625
S18	22.841	1.403	Open Manhole	1200	S1.011	21.438	300	S1.010	21.438
S19	24.537	1.425	Open Manhole	1200	S4.000	23.112	225		
S20	23.504	1.425	Open Manhole	1200	S4.001	22.079	225	S4.000	22.079
SPP3	22.895	1.000	Open Manhole	1200	S5.000	21.895	150		
SHBRAKE PP3	22.850	1.425	Open Manhole	1200	S4.002	21.425	225	S4.001	21.425
								S5.000	21.895
S23	22.467	1.475	Open Manhole	1350	S1.012	20.992	375	S1.011	20.992
								S4.002	21.425
S24	22.335	1.444	Open Manhole	1350	S1.013	20.892	450	S1.012	20.892
S25	22.229	1.357	Open Manhole	1350	S1.014	20.872	450	S1.013	20.872
S26	22.132	1.357	Open Manhole	1350	S1.015	20.775	450	S1.014	20.775
S27	22.602	1.876	Open Manhole	1350	S1.016	20.726	450	S1.015	20.726
S28	25.256	1.350	Open Manhole	1200	S6.000	23.906	150		
S29	25.560	1.789	Open Manhole	1200	S6.001	23.770	150	S6.000	23.770
S30	25.234	1.654	Open Manhole	1200	S6.002	23.580	150	S6.001	23.580
S31	23.530	1.350	Open Manhole	1200	S6.003	22.180	150	S6.002	22.180
SBASIN J	22.500	0.900	Open Manhole	1200	S6.004	21.600	150	S6.003	21.600
SHBRAKE BASIN J	22.500	1.086	Open Manhole	1200	S6.005	21.414	150	S6.004	21.414
SBASIN I	21.600	1.000	Open Manhole	1350	S1.017	20.600	450	S1.016	20.600
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Manhole Schedules for Storm

MH Name	MH CL (m)	MH Depth (m)	MH Connection	MH Diam., L*W (mm)	PN	Pipe Out Invert Level (m)	Diameter (mm)	PN	Pipes In Invert Level (m)	Diameter (mm)
SHBRAKE BASIN I	21.100	1.050	Open Manhole	1350	S1.018	20.050	300	S6.005	20.900	150
	20.300	0.418	Open Manhole	0		OUTFALL		S1.017	20.050	450
S								S1.018	19.882	300

MH Name	Manhole Easting (m)	Manhole Northing (m)	Intersection Easting (m)	Intersection Northing (m)	Manhole Access	Layout (North)
SPP1	449017.276	116204.217	449017.276	116204.217	Required	
SHBRAKE PP1	449017.901	116209.038	449017.901	116209.038	Required	
S3	448988.776	116225.812	448988.776	116225.812	Required	
S4	449000.622	116246.547	449000.622	116246.547	Required	
S5	448992.771	116251.037	448992.771	116251.037	Required	
SPP2	449059.454	116257.872	449059.454	116257.872	Required	
SHBRAKE PP2	449054.217	116251.472	449054.217	116251.472	Required	
S8	449031.533	116264.525	449031.533	116264.525	Required	
S9	449029.275	116268.836	449029.275	116268.836	Required	
S10	449010.637	116279.103	449010.637	116279.103	Required	
S11	449017.601	116291.679	449017.601	116291.679	Required	
S12	449021.778	116304.213	449021.778	116304.213	Required	



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MH Name	Manhole Easting (m)	Manhole Northing (m)	Intersection Easting (m)	Intersection Northing (m)	Manhole Access	Layout (North)
S13	449028.405	116315.288	449028.405	116315.288	Required	
S14	449077.743	116305.142	449077.743	116305.142	Required	
S15	449056.035	116318.221	449056.035	116318.221	Required	
S16	449036.410	116328.985	449036.410	116328.985	Required	
S17	449045.466	116346.481	449045.466	116346.481	Required	
S18	449052.354	116356.235	449052.354	116356.235	Required	
S19	449110.976	116349.600	449110.976	116349.600	Required	
S20	449086.876	116361.476	449086.876	116361.476	Required	
SPP3	449075.530	116372.816	449075.530	116372.816	Required	
SHBRAKE PP3	449070.626	116368.435	449070.626	116368.435	Required	
S23	449056.346	116373.540	449056.346	116373.540	Required	
S24	449059.135	116380.942	449059.135	116380.942	Required	
S25	449061.639	116387.330	449061.639	116387.330	Required	
S26	449060.142	116393.629	449060.142	116393.629	Required	
S27	449066.588	116409.712	449066.588	116409.712	Required	

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Manhole Schedules for Storm

MH Name	Manhole Easting (m)	Manhole Northing (m)	Intersection Easting (m)	Intersection Northing (m)	Manhole Access	Layout (North)
S28	449131.816	116327.692	449131.816	116327.692	Required	
S29	449141.616	116345.534	449141.616	116345.534	Required	
S30	449151.873	116372.185	449151.873	116372.185	Required	
S31	449099.208	116393.637	449099.208	116393.637	Required	
SBASIN J	449110.340	116418.843	449110.340	116418.843	Required	
SHBRAKE BASIN J	449085.669	116432.026	449085.669	116432.026	Required	
SBASIN I	449052.501	116443.750	449052.501	116443.750	Required	
SHBRAKE BASIN I	449007.623	116462.133	449007.623	116462.133	Required	
S	448995.517	116468.805			No Entry	

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PIPELINE SCHEDULES for Storm

Upstream Manhole

PN	Hyd Sect	Diam (mm)	MH Name	C.Level (m)	I.Level (m)	D.Depth (m)	MH Connection	MH DIAM., L*W (mm)
S1.000	o	150	SPP1	27.241	26.241	0.850	Open Manhole	1200
S1.001	o	225	SHBRAKE PP1	27.430	26.005	1.200	Open Manhole	1200
S1.002	o	225	S3	26.568	25.143	1.200	Open Manhole	1200
S1.003	o	225	S4	26.215	24.790	1.200	Open Manhole	1200
S1.004	o	225	S5	25.945	24.520	1.200	Open Manhole	1200
S2.000	o	150	SPP2	25.217	24.217	0.850	Open Manhole	1200
S2.001	o	225	SHBRAKE PP2	25.800	24.087	1.488	Open Manhole	1200
S2.002	o	225	S8	25.458	23.930	1.303	Open Manhole	1200
S2.003	o	225	S9	25.390	23.901	1.264	Open Manhole	1200
S1.005	o	300	S10	25.202	23.699	1.203	Open Manhole	1200
S1.006	o	300	S11	24.804	23.301	1.203	Open Manhole	1200
S1.007	o	300	S12	24.441	22.938	1.204	Open Manhole	1200
S1.008	o	300	S13	24.097	22.594	1.203	Open Manhole	1200
S3.000	o	225	S14	24.058	22.633	1.200	Open Manhole	1200
S3.001	o	225	S15	23.758	22.333	1.200	Open Manhole	1200
S1.009	o	300	S16	23.662	22.121	1.241	Open Manhole	1200
S1.010	o	300	S17	23.127	21.625	1.203	Open Manhole	1200

Downstream Manhole

PN	Length (m)	Slope (1:X)	MH Name	C.Level (m)	I.Level (m)	D.Depth (m)	MH Connection	MH DIAM., L*W (mm)
S1.000	4.862	30.2	SHBRAKE PP1	27.430	26.080	1.200	Open Manhole	1200
S1.001	33.610	39.0	S3	26.568	25.143	1.200	Open Manhole	1200
S1.002	23.881	67.7	S4	26.215	24.790	1.200	Open Manhole	1200
S1.003	9.044	33.5	S5	25.945	24.520	1.200	Open Manhole	1200
S1.004	33.270	44.6	S10	25.202	23.774	1.203	Open Manhole	1200
S2.000	8.269	150.0	SHBRAKE PP2	25.800	24.162	1.488	Open Manhole	1200
S2.001	26.172	167.0	S8	25.458	23.930	1.303	Open Manhole	1200
S2.002	4.867	167.0	S9	25.390	23.901	1.264	Open Manhole	1200
S2.003	21.279	167.0	S10	25.202	23.774	1.203	Open Manhole	1200
S1.005	14.376	36.1	S11	24.804	23.301	1.203	Open Manhole	1200
S1.006	13.212	36.4	S12	24.441	22.938	1.204	Open Manhole	1200
S1.007	12.906	37.5	S13	24.097	22.594	1.203	Open Manhole	1200
S1.008	15.865	33.5	S16	23.662	22.121	1.241	Open Manhole	1200
S3.000	25.344	84.5	S15	23.758	22.333	1.200	Open Manhole	1200
S3.001	22.383	162.9	S16	23.662	22.196	1.241	Open Manhole	1200
S1.009	19.700	39.7	S17	23.127	21.625	1.203	Open Manhole	1200
S1.010	11.941	63.9	S18	22.841	21.438	1.103	Open Manhole	1200

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PIPELINE SCHEDULES for Storm

Upstream Manhole

PN	Hyd Sect	Diam (mm)	MH Name	C.Level (m)	I.Level (m)	D.Depth (m)	MH Connection	MH DIAM., L*W (mm)
S1.011	o	300	S18	22.841	21.438	1.103	Open Manhole	1200
S4.000	o	225	S19	24.537	23.112	1.200	Open Manhole	1200
S4.001	o	225	S20	23.504	22.079	1.200	Open Manhole	1200
S5.000	o	150	SPP3	22.895	21.895	0.850	Open Manhole	1200
S4.002	o	225	SHBRAKE PP3	22.850	21.425	1.200	Open Manhole	1200
S1.012	o	375	S23	22.467	20.992	1.100	Open Manhole	1350
S1.013	o	450	S24	22.335	20.892	0.994	Open Manhole	1350
S1.014	o	450	S25	22.229	20.872	0.907	Open Manhole	1350
S1.015	o	450	S26	22.132	20.775	0.907	Open Manhole	1350
S1.016	o	450	S27	22.602	20.726	1.426	Open Manhole	1350
S6.000	o	150	S28	25.256	23.906	1.200	Open Manhole	1200
S6.001	o	150	S29	25.560	23.770	1.639	Open Manhole	1200
S6.002	o	150	S30	25.234	23.580	1.504	Open Manhole	1200
S6.003	o	150	S31	23.530	22.180	1.200	Open Manhole	1200
S6.004	o	150	SBASIN J	22.500	21.600	0.750	Open Manhole	1200
S6.005	o	150	SHBRAKE BASIN J	22.500	21.414	0.936	Open Manhole	1200

Downstream Manhole

PN	Length (m)	Slope (1:X)	MH Name	C.Level (m)	I.Level (m)	D.Depth (m)	MH Connection	MH DIAM., L*W (mm)
S1.011	17.760	47.9	S23	22.467	21.067	1.100	Open Manhole	1350
S4.000	26.867	26.0	S20	23.504	22.079	1.200	Open Manhole	1200
S4.001	17.677	27.0	SHBRAKE PP3	22.850	21.425	1.200	Open Manhole	1200
S5.000	6.576	16.6	SHBRAKE PP3	22.850	21.500	1.200	Open Manhole	1200
S4.002	15.165	53.6	S23	22.467	21.142	1.100	Open Manhole	1350
S1.012	7.910	313.0	S24	22.335	20.967	0.994	Open Manhole	1350
S1.013	6.861	350.0	S25	22.229	20.872	0.907	Open Manhole	1350
S1.014	6.475	66.8	S26	22.132	20.775	0.907	Open Manhole	1350
S1.015	17.327	350.0	S27	22.602	20.726	1.426	Open Manhole	1350
S1.016	36.838	292.4	SBASIN I	21.600	20.600	0.550	Open Manhole	1350
S6.000	20.356	150.0	S29	25.560	23.770	1.639	Open Manhole	1200
S6.001	28.557	150.0	S30	25.234	23.580	1.504	Open Manhole	1200
S6.002	56.866	40.6	S31	23.530	22.180	1.200	Open Manhole	1200
S6.003	27.554	47.5	SBASIN J	22.500	21.600	0.750	Open Manhole	1200
S6.004	27.972	150.4	SHBRAKE BASIN J	22.500	21.414	0.936	Open Manhole	1200
S6.005	35.180	68.5	SBASIN I	21.600	20.900	0.550	Open Manhole	1350

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PIPELINE SCHEDULES for Storm

Upstream Manhole

PN	Hyd Sect	Diam (mm)	MH Name	C.Level (m)	I.Level (m)	D.Depth (m)	MH Connection	MH DIAM., L*W (mm)
S1.017	o	450	SBASIN I	21.600	20.600	0.550	Open Manhole	1350
S1.018	o	300	SHBRAKE BASIN I	21.100	20.050	0.750	Open Manhole	1350

Downstream Manhole

PN	Length (m)	Slope (1:X)	MH Name	C.Level (m)	I.Level (m)	D.Depth (m)	MH Connection	MH DIAM., L*W (mm)
S1.017	48.497	88.2	SHBRAKE BASIN I	21.100	20.050	0.600	Open Manhole	1350
S1.018	13.823	82.3	S	20.300	19.882	0.118	Open Manhole	0

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Area Summary for Storm

Pipe Number	PIMP Type	PIMP Name	PIMP (%)	Gross Area (ha)	Imp. Area (ha)	Pipe Total (ha)
1.000	-	-	100	0.023	0.023	0.023
1.001	-	-	100	0.071	0.071	0.071
1.002	-	-	100	0.036	0.036	0.036
1.003	-	-	100	0.050	0.050	0.050
1.004	-	-	100	0.005	0.005	0.005
2.000	-	-	100	0.046	0.046	0.046
2.001	-	-	100	0.066	0.066	0.066
2.002	-	-	100	0.053	0.053	0.053
2.003	-	-	100	0.003	0.003	0.003
1.005	-	-	100	0.052	0.052	0.052
1.006	-	-	100	0.022	0.022	0.022
1.007	-	-	100	0.018	0.018	0.018
1.008	-	-	100	0.018	0.018	0.018
3.000	-	-	100	0.102	0.102	0.102
3.001	-	-	100	0.054	0.054	0.054
1.009	-	-	100	0.036	0.036	0.036
1.010	-	-	100	0.029	0.029	0.029
1.011	-	-	100	0.020	0.020	0.020
4.000	-	-	100	0.071	0.071	0.071
4.001	-	-	100	0.040	0.040	0.040
5.000	-	-	100	0.030	0.030	0.030
4.002	-	-	100	0.011	0.011	0.011
1.012	-	-	100	0.030	0.030	0.030
1.013	-	-	100	0.000	0.000	0.000
1.014	-	-	100	0.007	0.007	0.007
1.015	-	-	100	0.004	0.004	0.004
1.016	-	-	100	0.000	0.000	0.000
6.000	-	-	100	0.011	0.011	0.011
6.001	-	-	100	0.011	0.011	0.011
6.002	-	-	100	0.019	0.019	0.019
6.003	-	-	100	0.050	0.050	0.050
6.004	-	-	100	0.000	0.000	0.000
6.005	-	-	100	0.000	0.000	0.000
1.017	-	-	100	0.000	0.000	0.000
1.018	-	-	100	0.000	0.000	0.000
				Total	Total	Total
				0.988	0.988	0.988

Surcharged Outfall Details for Storm

Outfall Pipe Number	Outfall Name	C. Level (m)	I. Level (m)	Min I. Level (m)	D,L (mm)	W (mm)
S1.018	S	20.300	19.882	0.000	0	0
Datum (m) 0.000 Offset (mins) 0						

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Online Controls for Storm

Hydro-Brake® Optimum Manhole: SHBRAKE PP1, DS/PN: S1.001, Volume (m³): 1.7

Unit Reference MD-SHE-0068-2000-0936-2000
Design Head (m) 0.936
Design Flow (l/s) 2.0
Flush-Flo™ Calculated
Objective Minimise upstream storage
Application Surface
Sump Available Yes
Diameter (mm) 68
Invert Level (m) 26.005
Minimum Outlet Pipe Diameter (mm) 100
Suggested Manhole Diameter (mm) 1200

Control Points	Head (m)	Flow (l/s)
Design Point (Calculated)	0.936	2.0
Flush-Flo™	0.287	2.0
Kick-Flo®	0.586	1.6
Mean Flow over Head Range	-	1.8

The hydrological calculations have been based on the Head/Discharge relationship for the Hydro-Brake® Optimum as specified. Should another type of control device other than a Hydro-Brake Optimum® be utilised then these storage routing calculations will be invalidated

Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)
0.100	1.7	1.200	2.2	3.000	3.4	7.000	5.1
0.200	2.0	1.400	2.4	3.500	3.7	7.500	5.2
0.300	2.0	1.600	2.6	4.000	3.9	8.000	5.4
0.400	2.0	1.800	2.7	4.500	4.1	8.500	5.6
0.500	1.8	2.000	2.8	5.000	4.3	9.000	5.7
0.600	1.6	2.200	3.0	5.500	4.5	9.500	5.9
0.800	1.9	2.400	3.1	6.000	4.7		
1.000	2.1	2.600	3.2	6.500	4.9		

Orifice Manhole: SHBRAKE PP2, DS/PN: S2.001, Volume (m³): 2.1

Diameter (m) 0.045 Discharge Coefficient 0.600 Invert Level (m) 24.087

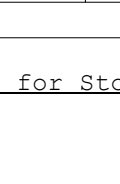
Hydro-Brake® Optimum Manhole: SHBRAKE PP3, DS/PN: S4.002, Volume (m³): 2.4

Unit Reference MD-SHE-0143-1000-1170-1000
Design Head (m) 1.170
Design Flow (l/s) 10.0
Flush-Flo™ Calculated
Objective Minimise upstream storage
Application Surface
Sump Available Yes
Diameter (mm) 143

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Hydro-Brake® Optimum Manhole: SHBRAKE PP3, DS/PN: S4.002, Volume (m³): 2.4																																																																										
Invert Level (m) 21.425 Minimum Outlet Pipe Diameter (mm) 225 Suggested Manhole Diameter (mm) 1200																																																																										
<table><tr><th>Control Points</th><th>Head (m)</th><th>Flow (l/s)</th></tr><tr><td>Design Point (Calculated)</td><td>1.170</td><td>10.0</td></tr><tr><td>Flush-Flo™</td><td>0.348</td><td>10.0</td></tr><tr><td>Kick-Flo®</td><td>0.763</td><td>8.2</td></tr><tr><td>Mean Flow over Head Range</td><td>-</td><td>8.6</td></tr></table>			Control Points	Head (m)	Flow (l/s)	Design Point (Calculated)	1.170	10.0	Flush-Flo™	0.348	10.0	Kick-Flo®	0.763	8.2	Mean Flow over Head Range	-	8.6																																																									
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<table><tr><th>Depth (m)</th><th>Flow (l/s)</th><th>Depth (m)</th><th>Flow (l/s)</th><th>Depth (m)</th><th>Flow (l/s)</th><th>Depth (m)</th><th>Flow (l/s)</th></tr><tr><td>0.100</td><td>5.2</td><td>1.200</td><td>10.1</td><td>3.000</td><td>15.6</td><td>7.000</td><td>23.4</td></tr><tr><td>0.200</td><td>9.5</td><td>1.400</td><td>10.9</td><td>3.500</td><td>16.8</td><td>7.500</td><td>24.2</td></tr><tr><td>0.300</td><td>10.0</td><td>1.600</td><td>11.6</td><td>4.000</td><td>17.9</td><td>8.000</td><td>25.0</td></tr><tr><td>0.400</td><td>10.0</td><td>1.800</td><td>12.2</td><td>4.500</td><td>18.9</td><td>8.500</td><td>25.7</td></tr><tr><td>0.500</td><td>9.8</td><td>2.000</td><td>12.9</td><td>5.000</td><td>19.9</td><td>9.000</td><td>26.4</td></tr><tr><td>0.600</td><td>9.5</td><td>2.200</td><td>13.5</td><td>5.500</td><td>20.9</td><td>9.500</td><td>27.1</td></tr><tr><td>0.800</td><td>8.4</td><td>2.400</td><td>14.0</td><td>6.000</td><td>21.7</td><td></td><td></td></tr><tr><td>1.000</td><td>9.3</td><td>2.600</td><td>14.6</td><td>6.500</td><td>22.6</td><td></td><td></td></tr></table>			Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	0.100	5.2	1.200	10.1	3.000	15.6	7.000	23.4	0.200	9.5	1.400	10.9	3.500	16.8	7.500	24.2	0.300	10.0	1.600	11.6	4.000	17.9	8.000	25.0	0.400	10.0	1.800	12.2	4.500	18.9	8.500	25.7	0.500	9.8	2.000	12.9	5.000	19.9	9.000	26.4	0.600	9.5	2.200	13.5	5.500	20.9	9.500	27.1	0.800	8.4	2.400	14.0	6.000	21.7			1.000	9.3	2.600	14.6	6.500	22.6		
Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)																																																																			
0.100	5.2	1.200	10.1	3.000	15.6	7.000	23.4																																																																			
0.200	9.5	1.400	10.9	3.500	16.8	7.500	24.2																																																																			
0.300	10.0	1.600	11.6	4.000	17.9	8.000	25.0																																																																			
0.400	10.0	1.800	12.2	4.500	18.9	8.500	25.7																																																																			
0.500	9.8	2.000	12.9	5.000	19.9	9.000	26.4																																																																			
0.600	9.5	2.200	13.5	5.500	20.9	9.500	27.1																																																																			
0.800	8.4	2.400	14.0	6.000	21.7																																																																					
1.000	9.3	2.600	14.6	6.500	22.6																																																																					
Hydro-Brake® Optimum Manhole: SHBRAKE BASIN J, DS/PN: S6.005, Volume (m³): 1.7																																																																										
Unit Reference MD-SHE-0046-1000-1086-1000 Design Head (m) 1.086 Design Flow (l/s) 1.0 Flush-Flo™ Calculated Objective Minimise upstream storage Application Surface Sump Available Yes Diameter (mm) 46 Invert Level (m) 21.414 Minimum Outlet Pipe Diameter (mm) 75 Suggested Manhole Diameter (mm) 1200																																																																										
<table><tr><th>Control Points</th><th>Head (m)</th><th>Flow (l/s)</th></tr><tr><td>Design Point (Calculated)</td><td>1.086</td><td>1.0</td></tr><tr><td>Flush-Flo™</td><td>0.202</td><td>0.8</td></tr><tr><td>Kick-Flo®</td><td>0.407</td><td>0.6</td></tr><tr><td>Mean Flow over Head Range</td><td>-</td><td>0.8</td></tr></table>			Control Points	Head (m)	Flow (l/s)	Design Point (Calculated)	1.086	1.0	Flush-Flo™	0.202	0.8	Kick-Flo®	0.407	0.6	Mean Flow over Head Range	-	0.8																																																									
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<u>Storage Structures for Storm</u> <u>Porous Car Park Manhole: SPP1, DS/PN: S1.000</u> <table><tr><td>Infiltration Coefficient Base (m/hr)</td><td>0.00000</td><td>Width (m)</td><td>13.6</td></tr><tr><td>Membrane Percolation (mm/hr)</td><td>1000</td><td>Length (m)</td><td>16.6</td></tr><tr><td>Max Percolation (l/s)</td><td>62.7</td><td>Slope (1:X)</td><td>70.0</td></tr><tr><td>Safety Factor</td><td>2.0</td><td>Depression Storage (mm)</td><td>5</td></tr><tr><td>Porosity</td><td>0.30</td><td>Evaporation (mm/day)</td><td>3</td></tr><tr><td>Invert Level (m)</td><td>26.241</td><td>Membrane Depth (mm)</td><td>0</td></tr></table> <u>Porous Car Park Manhole: SPP2, DS/PN: S2.000</u> <table><tr><td>Infiltration Coefficient Base (m/hr)</td><td>0.00000</td><td>Width (m)</td><td>20.4</td></tr><tr><td>Membrane Percolation (mm/hr)</td><td>1000</td><td>Length (m)</td><td>11.0</td></tr><tr><td>Max Percolation (l/s)</td><td>62.3</td><td>Slope (1:X)</td><td>300.0</td></tr><tr><td>Safety Factor</td><td>2.0</td><td>Depression Storage (mm)</td><td>5</td></tr><tr><td>Porosity</td><td>0.30</td><td>Evaporation (mm/day)</td><td>3</td></tr><tr><td>Invert Level (m)</td><td>24.217</td><td>Membrane Depth (mm)</td><td>0</td></tr></table> <u>Porous Car Park Manhole: SPP3, DS/PN: S5.000</u> <table><tr><td>Infiltration Coefficient Base (m/hr)</td><td>0.00000</td><td>Width (m)</td><td>10.0</td></tr><tr><td>Membrane Percolation (mm/hr)</td><td>1000</td><td>Length (m)</td><td>31.1</td></tr><tr><td>Max Percolation (l/s)</td><td>86.4</td><td>Slope (1:X)</td><td>50.0</td></tr><tr><td>Safety Factor</td><td>2.0</td><td>Depression Storage (mm)</td><td>5</td></tr><tr><td>Porosity</td><td>0.30</td><td>Evaporation (mm/day)</td><td>3</td></tr><tr><td>Invert Level (m)</td><td>21.895</td><td>Membrane Depth (mm)</td><td>0</td></tr></table> <u>Tank or Pond Manhole: SBASIN J, DS/PN: S6.004</u> Invert Level (m) 21.600 <table><tr><th>Depth (m)</th><th>Area (m²)</th><th>Depth (m)</th><th>Area (m²)</th></tr><tr><td>0.000</td><td>280.0</td><td>0.900</td><td>592.0</td></tr></table> <u>Tank or Pond Manhole: SBASIN I, DS/PN: S1.017</u> Invert Level (m) 20.600 <table><tr><th>Depth (m)</th><th>Area (m²)</th><th>Depth (m)</th><th>Area (m²)</th></tr><tr><td>0.000</td><td>919.0</td><td>1.000</td><td>1624.0</td></tr></table>			Infiltration Coefficient Base (m/hr)	0.00000	Width (m)	13.6	Membrane Percolation (mm/hr)	1000	Length (m)	16.6	Max Percolation (l/s)	62.7	Slope (1:X)	70.0	Safety Factor	2.0	Depression Storage (mm)	5	Porosity	0.30	Evaporation (mm/day)	3	Invert Level (m)	26.241	Membrane Depth (mm)	0	Infiltration Coefficient Base (m/hr)	0.00000	Width (m)	20.4	Membrane Percolation (mm/hr)	1000	Length (m)	11.0	Max Percolation (l/s)	62.3	Slope (1:X)	300.0	Safety Factor	2.0	Depression Storage (mm)	5	Porosity	0.30	Evaporation (mm/day)	3	Invert Level (m)	24.217	Membrane Depth (mm)	0	Infiltration Coefficient Base (m/hr)	0.00000	Width (m)	10.0	Membrane Percolation (mm/hr)	1000	Length (m)	31.1	Max Percolation (l/s)	86.4	Slope (1:X)	50.0	Safety Factor	2.0	Depression Storage (mm)	5	Porosity	0.30	Evaporation (mm/day)	3	Invert Level (m)	21.895	Membrane Depth (mm)	0	Depth (m)	Area (m²)	Depth (m)	Area (m²)	0.000	280.0	0.900	592.0	Depth (m)	Area (m²)	Depth (m)	Area (m²)	0.000	919.0	1.000	1624.0
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Summary of Critical Results by Maximum Level (Rank 1) for Storm

Simulation Criteria

Areal Reduction Factor	1.000	Additional Flow - % of Total Flow	0.000
Hot Start (mins)	0	MADD Factor * 10m³/ha Storage	2.000
Hot Start Level (mm)	0	Inlet Coeffiecient	0.800
Manhole Headloss Coeff (Global)	0.500	Flow per Person per Day (l/per/day)	0.000
Foul Sewage per hectare (l/s)	0.000		

Number of Input Hydrographs	0	Number of Storage Structures	5
Number of Online Controls	5	Number of Time/Area Diagrams	0
Number of Offline Controls	0	Number of Real Time Controls	0

Synthetic Rainfall Details

Rainfall Model	FSR	Ratio R	0.350
Region England and Wales Cv (Summer)			0.750
M5-60 (mm)	19.900 Cv (Winter)		0.840

Margin for Flood Risk Warning (mm)	300.0
Analysis Timestep	2.5 Second Increment (Extended)
DTS Status	ON
DVD Status	OFF
Inertia Status	OFF

Profile(s)	Summer and Winter
Duration(s) (mins)	15, 30, 60, 120, 180, 240, 360, 480, 600, 720, 960, 1440, 2160, 2880, 4320, 5760, 7200, 8640, 10080
Return Period(s) (years)	100
Climate Change (%)	40

PN	US/MH Name	Storm	Return Period	Climate Change	First (X) Surge	First (Y) Flood	First (Z) Overflow
S1.000	SPP1	180 Winter	100	+40%	100/15 Summer		
S1.001	SHBRAKE PP1	15 Winter	100	+40%	100/15 Summer		
S1.002	S3	15 Winter	100	+40%			
S1.003	S4	15 Winter	100	+40%			
S1.004	S5	15 Winter	100	+40%	100/15 Winter		
S2.000	SPP2	120 Winter	100	+40%	100/15 Summer		
S2.001	SHBRAKE PP2	15 Winter	100	+40%	100/15 Summer		
S2.002	S8	15 Winter	100	+40%	100/15 Summer		
S2.003	S9	15 Winter	100	+40%	100/15 Summer		
S1.005	S10	15 Winter	100	+40%	100/15 Summer		
S1.006	S11	15 Winter	100	+40%	100/15 Summer		
S1.007	S12	15 Winter	100	+40%	100/15 Summer		
S1.008	S13	15 Winter	100	+40%	100/15 Summer		
S3.000	S14	15 Winter	100	+40%	100/15 Summer	100/15 Summer	
S3.001	S15	15 Winter	100	+40%	100/15 Summer	100/15 Summer	
S1.009	S16	15 Winter	100	+40%	100/15 Summer	100/15 Winter	
S1.010	S17	15 Winter	100	+40%	100/15 Summer		
S1.011	S18	15 Winter	100	+40%	100/15 Summer		

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Summary of Critical Results by Maximum Level (Rank 1) for Storm

PN	US/MH Name	Overflow Act.	Water Surcharged			Flow / Cap.	Overflow (l/s)	Half Drain Time (mins)	Pipe Flow (l/s)
			Level (m)	Depth (m)	Volume (m³)				
S1.000	SPP1		26.915	0.524	0.000	0.09			2.2
S1.001	SHBRAKE PP1		26.961	0.731	0.000	0.03			2.0
S1.002	S3		25.245	-0.123	0.000	0.42			24.6
S1.003	S4		24.938	-0.077	0.000	0.77			56.2
S1.004	S5		24.746	0.001	0.000	0.80			58.8
S2.000	SPP2		24.861	0.494	0.000	0.28		124	3.5
S2.001	SHBRAKE PP2		24.913	0.601	0.000	0.09			3.4
S2.002	S8		24.504	0.349	0.000	1.08			31.3
S2.003	S9		24.479	0.353	0.000	0.88			32.0
S1.005	S10		24.420	0.421	0.000	0.72			111.3
S1.006	S11		24.272	0.672	0.000	0.75			112.6
S1.007	S12		24.104	0.866	0.000	0.75			109.7
S1.008	S13		23.911	1.018	0.000	0.72			117.1
S3.000	S14		24.059	1.201	0.312	0.98			51.1
S3.001	S15		23.765	1.207	6.835	1.74			64.8
S1.009	S16		23.662	1.241	0.086	1.08			165.3
S1.010	S17		23.114	1.190	0.000	1.65			178.2
S1.011	S18		22.611	0.874	0.000	1.35			185.8

PN	US/MH Name	Status	Level Exceeded
S1.000	SPP1	SURCHARGED	
S1.001	SHBRAKE PP1	SURCHARGED	
S1.002	S3	OK	
S1.003	S4	OK	
S1.004	S5	SURCHARGED	
S2.000	SPP2	SURCHARGED	
S2.001	SHBRAKE PP2	SURCHARGED	
S2.002	S8	SURCHARGED	
S2.003	S9	SURCHARGED	
S1.005	S10	SURCHARGED	
S1.006	S11	SURCHARGED	
S1.007	S12	SURCHARGED	
S1.008	S13	FLOOD RISK	
S3.000	S14	FLOOD	2
S3.001	S15	FLOOD	4
S1.009	S16	FLOOD	1
S1.010	S17	FLOOD RISK	
S1.011	S18	FLOOD RISK	

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File 2021-05-09 Basins I and...		Checked by			
Innovyze			Network 2020.1		
<u>Summary of Critical Results by Maximum Level (Rank 1) for Storm</u>					
PN	US/MH Name	Pipe Flow (l/s)	Status	Level Exceeded	
S4.002	SHBRAKE PP3	10.0	FLOOD		4
S1.012		S23 207.9	SURCHARGED		
S1.013		S24 208.1	SURCHARGED		
S1.014		S25 210.7	SURCHARGED		
S1.015		S26 210.7	SURCHARGED		
S1.016		S27 210.3	SURCHARGED		
S6.000		S28 6.9	OK		
S6.001		S29 12.2	SURCHARGED		
S6.002		S30 19.3	SURCHARGED		
S6.003		S31 41.0	FLOOD RISK		
S6.004	SBASIN J	1.4	SURCHARGED		
S6.005	SHBRAKE BASIN J	0.8	SURCHARGED		
S1.017	SBASIN I	43.6	OK		
S1.018	SHBRAKE BASIN I	29.5	FLOOD RISK		
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STORM SEWER DESIGN by the Modified Rational Method

Design Criteria for Storm

Pipe Sizes STANDARD Manhole Sizes STANDARD

FSR Rainfall Model - England and Wales

Return Period (years)	100	PIMP (%)	100
M5-60 (mm)	19.900	Add Flow / Climate Change (%)	0
Ratio R	0.350	Minimum Backdrop Height (m)	0.200
Maximum Rainfall (mm/hr)	50	Maximum Backdrop Height (m)	1.500
Maximum Time of Concentration (mins)	30	Min Design Depth for Optimisation (m)	1.200
Foul Sewage (l/s/ha)	0.000	Min Vel for Auto Design only (m/s)	1.00
Volumetric Runoff Coeff.	0.750	Min Slope for Optimisation (1:X)	500

Designed with Level Soffits

Time Area Diagram for Storm

Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)
0-4	0.088	4-8	1.045	8-12	0.218

Total Area Contributing (ha) = 1.351

Total Pipe Volume (m³) = 76.603

Network Design Table for Storm

« - Indicates pipe capacity < flow

PN	Length (m)	Fall (m)	Slope (1:X)	I.Area (ha)	T.E. (mins)	Base Flow (l/s)	k (mm)	HYD SECT	DIA (mm)	Section Type	Auto Design
S7.000	18.040	0.120	150.0	0.028	4.00	0.0	0.600	o	150	Pipe/Conduit	
S7.001	10.579	0.071	150.0	0.004	0.00	0.0	0.600	o	150	Pipe/Conduit	
S7.002	22.892	0.153	150.0	0.029	0.00	0.0	0.600	o	150	Pipe/Conduit	
S7.003	22.797	0.137	167.0	0.044	0.00	0.0	0.600	o	225	Pipe/Conduit	
S7.004	22.670	0.136	166.7	0.018	0.00	0.0	0.600	o	225	Pipe/Conduit	

Network Results Table

PN	Rain (mm/hr)	T.C. (mins)	US/IL (m)	Σ I.Area (ha)	Σ Base Flow (l/s)	Foul (l/s)	Add Flow (l/s)	Vel (m/s)	Cap (l/s)	Flow (l/s)
S7.000	50.00	4.37	22.118	0.028	0.0	0.0	0.0	0.82	14.5	3.8
S7.001	50.00	4.58	21.998	0.032	0.0	0.0	0.0	0.82	14.5	4.3
S7.002	50.00	5.05	21.927	0.061	0.0	0.0	0.0	0.82	14.5	8.3
S7.003	50.00	5.43	21.700	0.105	0.0	0.0	0.0	1.01	40.1	14.2
S7.004	50.00	5.80	21.563	0.123	0.0	0.0	0.0	1.01	40.2	16.7

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<u>Network Design Table for Storm</u>												
PN	Length (m)	Fall (m)	Slope (1:X)	I.Area (ha)	T.E. (mins)	Base Flow (l/s)	k (mm)	HYD SECT	DIA (mm)	Section	Type	Auto Design
S8.000	23.196	0.451	51.4	0.054	4.00	0.0	0.600	o	150	Pipe/Conduit		
S7.005	28.209	0.171	165.0	0.028	0.00	0.0	0.600	o	225	Pipe/Conduit		
S7.006	18.438	0.332	55.5	0.031	0.00	0.0	0.600	o	225	Pipe/Conduit		
S7.007	25.983	0.800	32.5	0.017	0.00	0.0	0.600	o	300	Pipe/Conduit		
S9.000	26.645	0.178	149.7	0.082	4.00	0.0	0.600	o	300	Pipe/Conduit		
S7.008	16.836	0.071	238.0	0.009	0.00	0.0	0.600	o	300	Pipe/Conduit		
S7.009	35.362	0.149	238.0	0.000	0.00	0.0	0.600	o	300	Pipe/Conduit		
S7.010	36.316	0.274	132.6	0.000	0.00	0.0	0.600	o	300	Pipe/Conduit		
S10.000	26.182	1.260	20.8	0.008	4.00	0.0	0.600	o	150	Pipe/Conduit		
S11.000	16.142	0.115	140.0	0.019	4.00	0.0	0.600	o	225	Pipe/Conduit		
S11.001	25.067	0.167	150.0	0.045	0.00	0.0	0.600	o	225	Pipe/Conduit		
S11.002	12.530	0.077	163.6	0.004	0.00	0.0	0.600	o	225	Pipe/Conduit		
S10.001	18.405	0.110	167.3	0.061	0.00	0.0	0.600	o	225	Pipe/Conduit		
S10.002	13.873	0.083	167.1	0.051	0.00	0.0	0.600	o	225	Pipe/Conduit		
S10.003	27.145	0.366	74.2	0.074	0.00	0.0	0.600	o	300	Pipe/Conduit		
<u>Network Results Table</u>												
PN	Rain (mm/hr)	T.C. (mins)	US/IL (m)	Σ I.Area (ha)	Σ Base Flow (l/s)	Foul (l/s)	Add Flow (l/s)	Vel (m/s)	Cap (l/s)	Flow (l/s)		
S8.000	50.00	4.27	21.953	0.054	0.0	0.0	0.0	1.41	24.8	7.3		
S7.005	50.00	6.26	21.427	0.205	0.0	0.0	0.0	1.02	40.4	27.8		
S7.006	50.00	6.44	21.256	0.236	0.0	0.0	0.0	1.76	69.9	32.0		
S7.007	50.00	6.59	20.849	0.253	0.0	0.0	0.0	2.77	195.7	34.3		
S9.000	50.00	4.35	20.227	0.082	0.0	0.0	0.0	1.28	90.7	11.1		
S7.008	50.00	6.87	20.049	0.344	0.0	0.0	0.0	1.01	71.7	46.6		
S7.009	50.00	7.45	19.978	0.344	0.0	0.0	0.0	1.01	71.7	46.6		
S7.010	50.00	7.90	19.830	0.344	0.0	0.0	0.0	1.36	96.4	46.6		
S10.000	50.00	4.20	21.525	0.008	0.0	0.0	0.0	2.22	39.2	1.1		
S11.000	50.00	4.24	20.549	0.019	0.0	0.0	0.0	1.10	43.9	2.6		
S11.001	50.00	4.64	20.434	0.064	0.0	0.0	0.0	1.07	42.4	8.7		
S11.002	50.00	4.84	20.267	0.068	0.0	0.0	0.0	1.02	40.5	9.2		
S10.001	50.00	5.15	20.190	0.137	0.0	0.0	0.0	1.01	40.1	18.6		
S10.002	50.00	5.37	20.080	0.188	0.0	0.0	0.0	1.01	40.1	25.5		
S10.003	50.00	5.62	19.922	0.262	0.0	0.0	0.0	1.83	129.2	35.5		
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PN

Length
(m)

Fall
(m)

Slope
(1:X)

I.Area
(ha)

T.E.
(mins)

Base
Flow (l/s)

k
(mm)

HYD
SECT

DIA
(mm)

Section

Type

Auto
Design

S7.011

34.212

0.109

313.0

0.043

0.00

0.0

0.600

o

375

Pipe/Conduit



S7.012

46.154

0.234

197.2

0.000

0.00

0.0

0.600

o

375

Pipe/Conduit



S12.000

16.586

0.111

149.4

0.037

4.00

0.0

0.600

o

225

Pipe/Conduit



S12.001

38.679

1.308

29.6

0.038

0.00

0.0

0.600

o

225

Pipe/Conduit



S13.000

19.980

1.113

18.0

0.047

4.00

0.0

0.600

o

150

Pipe/Conduit



S14.000

33.369

0.334

99.9

0.070

4.00

0.0

0.600

o

300

Pipe/Conduit



S12.002

5.834

0.035

167.0

0.060

0.00

0.0

0.600

o

300

Pipe/Conduit



S12.003

40.197

0.241

167.0

0.000

0.00

0.0

0.600

o

300

Pipe/Conduit



S12.004

23.880

0.239

100.0

0.071

0.00

0.0

0.600

o

300

Pipe/Conduit



S12.005

7.963

0.053

150.0

0.017

0.00

0.0

0.600

o

300

Pipe/Conduit



S12.006

24.831

0.104

238.0

0.000

0.00

0.0

0.600

o

300

Pipe/Conduit



S12.007

5.947

0.025

238.0

0.060

0.00

0.0

0.600

o

300

Pipe/Conduit



S12.008

6.270

0.117

53.6

0.000

0.00

0.0

0.600

o

300

Pipe/Conduit



S15.000

4.196

0.250

16.8

0.041

4.00

0.0

0.600

o

150

Pipe/Conduit



S15.001

15.978

0.107

150.0

0.000

0.00

0.0

0.600

o

150

Pipe/Conduit



S15.002

36.799

0.220

167.0

0.045

0.00

0.0

0.600

o

225

Pipe/Conduit



PN

Rain
(mm/hr)

T.C.
(mins)

US/IL
(m)

Σ I.Area
(ha)

Σ Base
Flow (l/s)

Foul
(l/s)

Add Flow
(l/s)

Vel
(m/s)

Cap
(l/s)

Flow
(l/s)

S7.011

50.00

8.46

19.481

0.649

0.0

0.0

0.0

1.02

112.5

87.9

S7.012

50.00

9.05

19.371

0.649

0.0

0.0

0.0

1.29

142.1

87.9

S12.000

50.00

4.26

21.708

0.037

0.0

0.0

0.0

1.07

42.4

5.0

S12.001

50.00

4.53

21.597

0.075

0.0

0.0

0.0

2.41

96.0

10.2

S13.000

50.00

4.14

21.477

0.047

0.0

0.0

0.0

2.39

42.2

6.4

S14.000

50.00

4.35

20.548

0.070

0.0

0.0

0.0

1.57

111.2

9.5

S12.002

50.00

4.61

20.214

0.252

0.0

0.0

0.0

1.21

85.8

34.1

S12.003

50.00

5.16

20.179

0.252

0.0

0.0

0.0

1.21

85.8

34.1

S12.004

50.00

5.41

19.938

0.323

0.0

0.0

0.0

1.57

111.1

43.7

S12.005

50.00

5.51

19.700

0.340

0.0

0.0

0.0

1.28

90.6

46.0

S12.006

50.00

5.92

19.646

0.340

0.0

0.0

0.0

1.01

71.7

46.0

S12.007

50.00

6.02

19.542

0.400

0.0

0.0

0.0

1.01

71.7

54.2

S12.008

50.00

6.07

19.517

0.400

0.0

0.0

0.0

2.15

152.1

54.2

S15.000

50.00

4.03

20.500

0.041

0.0

0.0

0.0

2.47

43.7

5.6

S15.001

50.00

4.35

20.250

0.041

0.0

0.0

0.0

0.82

14.5

5.6

S15.002

50.00

4.96

20.068

0.086

0.0

0.0

0.0

1.01

40.1

11.6

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Manhole Schedules for Storm

MH Name	MH CL (m)	MH Depth (m)	MH Connection	MH Diam., L*W (mm)	PN	Pipe Out Invert Level (m)	Diameter (mm)	PN	Pipes In Invert Level (m)	Diameter (mm)	Backdrop (mm)
S36	23.468	1.350	Open Manhole	1200	S7.000	22.118	150				
S37	23.943	1.946	Open Manhole	1200	S7.001	21.998	150	S7.000	21.998	150	
S38	24.213	2.286	Open Manhole	1200	S7.002	21.927	150	S7.001	21.927	150	
S39	24.833	3.133	Open Manhole	1200	S7.003	21.700	225	S7.002	21.775	150	
S40	23.947	2.384	Open Manhole	1200	S7.004	21.563	225	S7.003	21.563	225	
S41	23.303	1.350	Open Manhole	1200	S8.000	21.953	150				
S42	23.580	2.153	Open Manhole	1200	S7.005	21.427	225	S7.004	21.427	225	
								S8.000	21.502	150	
S43	22.681	1.425	Open Manhole	1200	S7.006	21.256	225	S7.005	21.256	225	
S44	22.349	1.500	Open Manhole	1200	S7.007	20.849	300	S7.006	20.924	225	
S45	21.727	1.500	Open Manhole	1200	S9.000	20.227	300				
S46	22.073	2.024	Open Manhole	1200	S7.008	20.049	300	S7.007	20.049	300	
								S9.000	20.049	300	
S47	22.299	2.320	Open Manhole	1200	S7.009	19.978	300	S7.008	19.978	300	
S48	21.841	2.011	Open Manhole	1200	S7.010	19.830	300	S7.009	19.830	300	
S49	22.875	1.350	Open Manhole	1200	S10.000	21.525	150				
S50	21.974	1.425	Open Manhole	1200	S11.000	20.549	225				
S51	22.360	1.926	Open Manhole	1200	S11.001	20.434	225	S11.000	20.434	225	
S52	22.450	2.184	Open Manhole	1200	S11.002	20.267	225	S11.001	20.267	225	
S53	22.420	2.230	Open Manhole	1200	S10.001	20.190	225	S10.000	20.265	150	
								S11.002	20.190	225	
S54	22.000	1.920	Open Manhole	1200	S10.002	20.080	225	S10.001	20.080	225	
S55	21.700	1.778	Open Manhole	1200	S10.003	19.922	300	S10.002	19.997	225	
S56	21.414	1.933	Open Manhole	1350	S7.011	19.481	375	S7.010	19.556	300	
								S10.003	19.556	300	
S57	21.232	1.861	Open Manhole	1350	S7.012	19.371	375	S7.011	19.371	375	
S58	23.500	1.792	Open Manhole	1200	S12.000	21.708	225				
S59	23.150	1.553	Open Manhole	1200	S12.001	21.597	225	S12.000	21.597	225	
S60	22.827	1.350	Open Manhole	1200	S13.000	21.477	150				
S61	22.048	1.500	Open Manhole	1200	S14.000	20.548	300				
S62	22.556	2.342	Open Manhole	1200	S12.002	20.214	300	S12.001	20.289	225	
								S13.000	20.364	150	
								S14.000	20.214	300	
S63	22.335	2.156	Open Manhole	1200	S12.003	20.179	300	S12.002	20.179	300	
S64	21.617	1.679	Open Manhole	1200	S12.004	19.938	300	S12.003	19.938	300	
S65	21.180	1.480	Open Manhole	1200	S12.005	19.700	300	S12.004	19.700	300	
S66	21.059	1.413	Open Manhole	1200	S12.006	19.646	300	S12.005	19.646	300	
S67	20.921	1.379	Open Manhole	1200	S12.007	19.542	300	S12.006	19.542	300	

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Manhole Schedules for Storm

MH Name	MH CL (m)	MH Depth (m)	MH Connection	MH Diam.,L*W (mm)	PN	Pipe Out Invert Level (m)	Diameter (mm)	PN	Pipes In Invert Level (m)	Diameter (mm)
S68	20.978	1.461	Open Manhole	1200	S12.008	19.517	300	S12.007	19.517	300
SPP4	21.300	0.800	Open Manhole	1200	S15.000	20.500	150			
SHBRAKE PP4	21.300	1.050	Open Manhole	1200	S15.001	20.250	150	S15.000	20.250	150
S71	21.220	1.152	Open Manhole	1200	S15.002	20.068	225	S15.001	20.143	150
S72	20.976	1.128	Open Manhole	1200	S15.003	19.848	225	S15.002	19.848	225
S73	20.965	1.167	Open Manhole	1200	S15.004	19.798	225	S15.003	19.798	225
S74	21.132	1.518	Open Manhole	1200	S15.005	19.614	300	S15.004	19.689	225
S75	21.207	1.730	Open Manhole	1200	S15.006	19.477	300	S15.005	19.477	300
S76	21.121	1.689	Open Manhole	1200	S15.007	19.432	300	S15.006	19.432	300
S77	21.033	1.707	Open Manhole	1350	S12.009	19.325	375	S12.008	19.400	300
								S15.007	19.400	300
S78	21.104	1.967	Open Manhole	1350	S7.013	19.292	450	S7.012	19.137	375
								S12.009	19.292	375
S79	21.080	1.921	Open Manhole	1350	S7.014	19.159	450	S7.013	19.159	450
S80	21.111	2.112	Open Manhole	1350	S7.015	18.999	450	S7.014	18.999	450
SBASIN K	20.100	1.200	Open Manhole	1350	S7.016	18.900	450	S7.015	18.900	450
SHBRAKE BASIN K	19.600	0.936	Open Manhole	1350	S7.017	18.664	450	S7.016	18.664	450
S	18.600	0.550	Open Manhole	0		OUTFALL		S7.017	18.050	450

MH Name	Manhole Easting (m)	Manhole Northing (m)	Intersection Easting (m)	Intersection Northing (m)	Manhole Access	Layout (North)
S36	448992.587	116327.978	448992.587	116327.978	Required	
S37	448985.857	116311.241	448985.857	116311.241	Required	
S38	448989.434	116301.285	448989.434	116301.285	Required	
S39	449008.038	116287.947	449008.038	116287.947	Required	
S40	449019.154	116307.850	449019.154	116307.850	Required	
S41	449007.530	116337.460	449007.530	116337.460	Required	



Micro
Drainage

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Network 2020.1

MH Name	Manhole Easting (m)	Manhole Northing (m)	Intersection Easting (m)	Intersection Northing (m)	Manhole Access	Layout (North)
S72	448845.533	116480.480	448845.533	116480.480	Required	
S73	448853.466	116482.884	448853.466	116482.884	Required	
S74	448870.356	116475.897	448870.356	116475.897	Required	
S75	448900.346	116462.951	448900.346	116462.951	Required	
S76	448909.422	116457.420	448909.422	116457.420	Required	
S77	448916.514	116454.834	448916.514	116454.834	Required	
S78	448920.532	116464.336	448920.532	116464.336	Required	
S79	448873.150	116483.867	448873.150	116483.867	Required	
S80	448815.996	116506.943	448815.996	116506.943	Required	
SBASIN K	448823.779	116535.986	448823.779	116535.986	Required	
SHBRAKE BASIN K	448825.024	116552.444	448825.024	116552.444	Required	
S	448828.056	116557.689			No Entry	

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Innovyze	Network 2020.1	

PIPELINE SCHEDULES for Storm

Upstream Manhole

PN	Hyd Sect	Diam (mm)	MH Name	C.Level (m)	I.Level (m)	D.Depth (m)	MH Connection	MH DIAM., L*W (mm)
S7.000	o	150	S36	23.468	22.118	1.200	Open Manhole	1200
S7.001	o	150	S37	23.943	21.998	1.796	Open Manhole	1200
S7.002	o	150	S38	24.213	21.927	2.136	Open Manhole	1200
S7.003	o	225	S39	24.833	21.700	2.908	Open Manhole	1200
S7.004	o	225	S40	23.947	21.563	2.159	Open Manhole	1200
S8.000	o	150	S41	23.303	21.953	1.200	Open Manhole	1200
S7.005	o	225	S42	23.580	21.427	1.928	Open Manhole	1200
S7.006	o	225	S43	22.681	21.256	1.200	Open Manhole	1200
S7.007	o	300	S44	22.349	20.849	1.200	Open Manhole	1200
S9.000	o	300	S45	21.727	20.227	1.200	Open Manhole	1200
S7.008	o	300	S46	22.073	20.049	1.724	Open Manhole	1200
S7.009	o	300	S47	22.299	19.978	2.020	Open Manhole	1200
S7.010	o	300	S48	21.841	19.830	1.711	Open Manhole	1200
S10.000	o	150	S49	22.875	21.525	1.200	Open Manhole	1200
S11.000	o	225	S50	21.974	20.549	1.200	Open Manhole	1200

Downstream Manhole

PN	Length (m)	Slope (1:X)	MH Name	C.Level (m)	I.Level (m)	D.Depth (m)	MH Connection	MH DIAM., L*W (mm)
S7.000	18.040	150.0	S37	23.943	21.998	1.796	Open Manhole	1200
S7.001	10.579	150.0	S38	24.213	21.927	2.136	Open Manhole	1200
S7.002	22.892	150.0	S39	24.833	21.775	2.908	Open Manhole	1200
S7.003	22.797	167.0	S40	23.947	21.563	2.159	Open Manhole	1200
S7.004	22.670	166.7	S42	23.580	21.427	1.928	Open Manhole	1200
S8.000	23.196	51.4	S42	23.580	21.502	1.928	Open Manhole	1200
S7.005	28.209	165.0	S43	22.681	21.256	1.200	Open Manhole	1200
S7.006	18.438	55.5	S44	22.349	20.924	1.200	Open Manhole	1200
S7.007	25.983	32.5	S46	22.073	20.049	1.724	Open Manhole	1200
S9.000	26.645	149.7	S46	22.073	20.049	1.724	Open Manhole	1200
S7.008	16.836	238.0	S47	22.299	19.978	2.020	Open Manhole	1200
S7.009	35.362	238.0	S48	21.841	19.830	1.711	Open Manhole	1200
S7.010	36.316	132.6	S56	21.414	19.556	1.558	Open Manhole	1350
S10.000	26.182	20.8	S53	22.420	20.265	2.005	Open Manhole	1200
S11.000	16.142	140.0	S51	22.360	20.434	1.701	Open Manhole	1200

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PIPELINE SCHEDULES for Storm

Upstream Manhole

PN	Hyd Sect	Diam (mm)	MH Name	C.Level (m)	I.Level (m)	D.Depth (m)	MH Connection	MH DIAM., L*W (mm)
S11.001	o	225	S51	22.360	20.434	1.701	Open Manhole	1200
S11.002	o	225	S52	22.450	20.267	1.959	Open Manhole	1200
S10.001	o	225	S53	22.420	20.190	2.005	Open Manhole	1200
S10.002	o	225	S54	22.000	20.080	1.695	Open Manhole	1200
S10.003	o	300	S55	21.700	19.922	1.478	Open Manhole	1200
S7.011	o	375	S56	21.414	19.481	1.558	Open Manhole	1350
S7.012	o	375	S57	21.232	19.371	1.486	Open Manhole	1350
S12.000	o	225	S58	23.500	21.708	1.567	Open Manhole	1200
S12.001	o	225	S59	23.150	21.597	1.328	Open Manhole	1200
S13.000	o	150	S60	22.827	21.477	1.200	Open Manhole	1200
S14.000	o	300	S61	22.048	20.548	1.200	Open Manhole	1200
S12.002	o	300	S62	22.556	20.214	2.042	Open Manhole	1200
S12.003	o	300	S63	22.335	20.179	1.856	Open Manhole	1200
S12.004	o	300	S64	21.617	19.938	1.379	Open Manhole	1200
S12.005	o	300	S65	21.180	19.700	1.180	Open Manhole	1200

Downstream Manhole

PN	Length (m)	Slope (1:X)	MH Name	C.Level (m)	I.Level (m)	D.Depth (m)	MH Connection	MH DIAM., L*W (mm)
S11.001	25.067	150.0	S52	22.450	20.267	1.959	Open Manhole	1200
S11.002	12.530	163.6	S53	22.420	20.190	2.005	Open Manhole	1200
S10.001	18.405	167.3	S54	22.000	20.080	1.695	Open Manhole	1200
S10.002	13.873	167.1	S55	21.700	19.997	1.478	Open Manhole	1200
S10.003	27.145	74.2	S56	21.414	19.556	1.558	Open Manhole	1350
S7.011	34.212	313.0	S57	21.232	19.371	1.486	Open Manhole	1350
S7.012	46.154	197.2	S78	21.104	19.137	1.592	Open Manhole	1350
S12.000	16.586	149.4	S59	23.150	21.597	1.328	Open Manhole	1200
S12.001	38.679	29.6	S62	22.556	20.289	2.042	Open Manhole	1200
S13.000	19.980	18.0	S62	22.556	20.364	2.042	Open Manhole	1200
S14.000	33.369	99.9	S62	22.556	20.214	2.042	Open Manhole	1200
S12.002	5.834	167.0	S63	22.335	20.179	1.856	Open Manhole	1200
S12.003	40.197	167.0	S64	21.617	19.938	1.379	Open Manhole	1200
S12.004	23.880	100.0	S65	21.180	19.700	1.180	Open Manhole	1200
S12.005	7.963	150.0	S66	21.059	19.646	1.113	Open Manhole	1200

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Innovyze		Network 2020.1				
<u>Area Summary for Storm</u>						
Pipe Number	PIMP Type	PIMP Name	PIMP (%)	Gross Area (ha)	Imp. Area (ha)	Pipe Total (ha)
7.000	-	-	100	0.028	0.028	0.028
7.001	-	-	100	0.004	0.004	0.004
7.002	-	-	100	0.029	0.029	0.029
7.003	-	-	100	0.044	0.044	0.044
7.004	-	-	100	0.018	0.018	0.018
8.000	-	-	100	0.054	0.054	0.054
7.005	-	-	100	0.028	0.028	0.028
7.006	-	-	100	0.031	0.031	0.031
7.007	-	-	100	0.017	0.017	0.017
9.000	-	-	100	0.082	0.082	0.082
7.008	-	-	100	0.009	0.009	0.009
7.009	-	-	100	0.000	0.000	0.000
7.010	-	-	100	0.000	0.000	0.000
10.000	-	-	100	0.008	0.008	0.008
11.000	-	-	100	0.019	0.019	0.019
11.001	-	-	100	0.045	0.045	0.045
11.002	-	-	100	0.004	0.004	0.004
10.001	-	-	100	0.061	0.061	0.061
10.002	-	-	100	0.051	0.051	0.051
10.003	-	-	100	0.074	0.074	0.074
7.011	-	-	100	0.043	0.043	0.043
7.012	-	-	100	0.000	0.000	0.000
12.000	-	-	100	0.037	0.037	0.037
12.001	-	-	100	0.038	0.038	0.038
13.000	-	-	100	0.047	0.047	0.047
14.000	-	-	100	0.070	0.070	0.070
12.002	-	-	100	0.060	0.060	0.060
12.003	-	-	100	0.000	0.000	0.000
12.004	-	-	100	0.071	0.071	0.071
12.005	-	-	100	0.017	0.017	0.017
12.006	-	-	100	0.000	0.000	0.000
12.007	-	-	100	0.060	0.060	0.060
12.008	-	-	100	0.000	0.000	0.000
15.000	-	-	100	0.041	0.041	0.041
15.001	-	-	100	0.000	0.000	0.000
15.002	-	-	100	0.045	0.045	0.045
15.003	-	-	100	0.064	0.064	0.064
15.004	-	-	100	0.023	0.023	0.023
15.005	-	-	100	0.046	0.046	0.046
15.006	-	-	100	0.033	0.033	0.033
15.007	-	-	100	0.000	0.000	0.000
12.009	-	-	100	0.033	0.033	0.033
7.013	-	-	100	0.000	0.000	0.000
7.014	-	-	100	0.000	0.000	0.000
7.015	-	-	100	0.017	0.017	0.017
7.016	-	-	100	0.000	0.000	0.000
7.017	-	-	100	0.000	0.000	0.000
				Total	Total	Total
				1.351	1.351	1.351
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Surcharged Outfall Details for Storm

Outfall Pipe Number	Outfall Name	C. Level (m)	I. Level (m)	Min I. Level (m)	D,L (mm)	W (mm)
S7.017	S	18.600	18.050	18.050	0	0

Datum (m) 0.000 Offset (mins) 0

Time (mins)	Depth (m)	Time (mins)	Depth (m)	Time (mins)	Depth (m)	Time (mins)	Depth (m)	Time (mins)	Depth (m)
1440	18.400	5760	18.400	10080	18.400	14400	18.400	18720	18.400
2880	18.400	7200	18.400	11520	18.400	15840	18.400	20160	18.400
4320	18.400	8640	18.400	12960	18.400	17280	18.400		

Simulation Criteria for Storm

Volumetric Runoff Coeff 0.750 Additional Flow - % of Total Flow 0.000
 Areal Reduction Factor 1.000 MADD Factor * 10m³/ha Storage 2.000
 Hot Start (mins) 0 Inlet Coefficient 0.800
 Hot Start Level (mm) 0 Flow per Person per Day (l/per/day) 0.000
 Manhole Headloss Coeff (Global) 0.500 Run Time (mins) 60
 Foul Sewage per hectare (l/s) 0.000 Output Interval (mins) 1

Number of Input Hydrographs 0 Number of Storage Structures 2
 Number of Online Controls 2 Number of Time/Area Diagrams 0
 Number of Offline Controls 0 Number of Real Time Controls 0

Synthetic Rainfall Details

Rainfall Model	FSR	Profile Type	Summer
Return Period (years)	100	Cv (Summer)	0.750
Region	England and Wales	Cv (Winter)	0.840
M5-60 (mm)	19.900	Storm Duration (mins)	30
Ratio R	0.350		

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Online Controls for Storm

Hydro-Brake® Optimum Manhole: SHBRAKE PP4, DS/PN: S15.001, Volume (m³): 1.2

Unit Reference	MD-SHE-0061-1500-0750-1500
Design Head (m)	0.750
Design Flow (l/s)	1.5
Flush-Flo™	Calculated
Objective	Minimise upstream storage
Application	Surface
Sump Available	Yes
Diameter (mm)	61
Invert Level (m)	20.250
Minimum Outlet Pipe Diameter (mm)	75
Suggested Manhole Diameter (mm)	1200

Control Points	Head (m)	Flow (l/s)
Design Point (Calculated)	0.750	1.5
Flush-Flo™	0.231	1.5
Kick-Flo®	0.479	1.2
Mean Flow over Head Range	-	1.3

The hydrological calculations have been based on the Head/Discharge relationship for the Hydro-Brake® Optimum as specified. Should another type of control device other than a Hydro-Brake Optimum® be utilised then these storage routing calculations will be invalidated

Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)
0.100	1.3	1.200	1.9	3.000	2.8	7.000	4.2
0.200	1.5	1.400	2.0	3.500	3.0	7.500	4.3
0.300	1.5	1.600	2.1	4.000	3.2	8.000	4.5
0.400	1.4	1.800	2.2	4.500	3.4	8.500	4.6
0.500	1.3	2.000	2.3	5.000	3.6	9.000	4.7
0.600	1.4	2.200	2.4	5.500	3.7	9.500	4.9
0.800	1.5	2.400	2.5	6.000	3.9		
1.000	1.7	2.600	2.6	6.500	4.0		

Hydro-Brake® Optimum Manhole: SBASIN K, DS/PN: S7.016, Volume (m³): 6.3

Unit Reference	MD-SHE-0165-1360-1200-1360
Design Head (m)	1.200
Design Flow (l/s)	13.6
Flush-Flo™	Calculated
Objective	Minimise upstream storage
Application	Surface
Sump Available	Yes
Diameter (mm)	165
Invert Level (m)	18.900
Minimum Outlet Pipe Diameter (mm)	225
Suggested Manhole Diameter (mm)	1500

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<u>Hydro-Brake® Optimum Manhole: SBASIN K, DS/PN: S7.016, Volume (m³): 6.3</u>																																																																										
<table><tr><td>Control Points</td><td>Head (m)</td><td>Flow (l/s)</td></tr><tr><td>Design Point (Calculated)</td><td>1.200</td><td>13.6</td></tr><tr><td>Flush-Flo™</td><td>0.364</td><td>13.6</td></tr><tr><td>Kick-Flo®</td><td>0.801</td><td>11.2</td></tr><tr><td>Mean Flow over Head Range</td><td>-</td><td>11.7</td></tr></table>			Control Points	Head (m)	Flow (l/s)	Design Point (Calculated)	1.200	13.6	Flush-Flo™	0.364	13.6	Kick-Flo®	0.801	11.2	Mean Flow over Head Range	-	11.7																																																									
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The hydrological calculations have been based on the Head/Discharge relationship for the Hydro-Brake® Optimum as specified. Should another type of control device other than a Hydro-Brake Optimum® be utilised then these storage routing calculations will be invalidated																																																																										
<table><tr><td>Depth (m)</td><td>Flow (l/s)</td><td>Depth (m)</td><td>Flow (l/s)</td><td>Depth (m)</td><td>Flow (l/s)</td><td>Depth (m)</td><td>Flow (l/s)</td></tr><tr><td>0.100</td><td>5.9</td><td>1.200</td><td>13.6</td><td>3.000</td><td>21.0</td><td>7.000</td><td>31.6</td></tr><tr><td>0.200</td><td>12.8</td><td>1.400</td><td>14.6</td><td>3.500</td><td>22.6</td><td>7.500</td><td>32.7</td></tr><tr><td>0.300</td><td>13.5</td><td>1.600</td><td>15.6</td><td>4.000</td><td>24.1</td><td>8.000</td><td>33.7</td></tr><tr><td>0.400</td><td>13.6</td><td>1.800</td><td>16.5</td><td>4.500</td><td>25.5</td><td>8.500</td><td>34.7</td></tr><tr><td>0.500</td><td>13.4</td><td>2.000</td><td>17.3</td><td>5.000</td><td>26.9</td><td>9.000</td><td>35.7</td></tr><tr><td>0.600</td><td>13.1</td><td>2.200</td><td>18.1</td><td>5.500</td><td>28.1</td><td>9.500</td><td>36.6</td></tr><tr><td>0.800</td><td>11.3</td><td>2.400</td><td>18.9</td><td>6.000</td><td>29.3</td><td></td><td></td></tr><tr><td>1.000</td><td>12.5</td><td>2.600</td><td>19.6</td><td>6.500</td><td>30.5</td><td></td><td></td></tr></table>			Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	0.100	5.9	1.200	13.6	3.000	21.0	7.000	31.6	0.200	12.8	1.400	14.6	3.500	22.6	7.500	32.7	0.300	13.5	1.600	15.6	4.000	24.1	8.000	33.7	0.400	13.6	1.800	16.5	4.500	25.5	8.500	34.7	0.500	13.4	2.000	17.3	5.000	26.9	9.000	35.7	0.600	13.1	2.200	18.1	5.500	28.1	9.500	36.6	0.800	11.3	2.400	18.9	6.000	29.3			1.000	12.5	2.600	19.6	6.500	30.5		
Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)																																																																			
0.100	5.9	1.200	13.6	3.000	21.0	7.000	31.6																																																																			
0.200	12.8	1.400	14.6	3.500	22.6	7.500	32.7																																																																			
0.300	13.5	1.600	15.6	4.000	24.1	8.000	33.7																																																																			
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0.500	13.4	2.000	17.3	5.000	26.9	9.000	35.7																																																																			
0.600	13.1	2.200	18.1	5.500	28.1	9.500	36.6																																																																			
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Storage Structures for Storm

Porous Car Park Manhole: SPP4, DS/PN: S15.000

Infiltration Coefficient Base (m/hr)	0.00000	Width (m)	10.0
Membrane Percolation (mm/hr)	1000	Length (m)	31.0
Max Percolation (l/s)	86.1	Slope (1:X)	100.0
Safety Factor	2.0	Depression Storage (mm)	5
Porosity	0.30	Evaporation (mm/day)	3
Invert Level (m)	20.500	Membrane Depth (mm)	0

Tank or Pond Manhole: SBASIN K, DS/PN: S7.016

Invert Level (m) 18.900

Depth (m)	Area (m ²)	Depth (m)	Area (m ²)
0.000	807.0	1.200	1502.0

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Micro Drainage

Summary of Critical Results by Maximum Level (Rank 1) for Storm

PN	US/MH Name	Storm	Return Period	Climate Change	First (X) Surcharge	First (Y) Flood	First (Z) Overflow
S11.001	S51	15 Winter	100	+40%	100/15 Summer		
S11.002	S52	15 Winter	100	+40%	100/15 Summer		
S10.001	S53	15 Winter	100	+40%	100/15 Summer		
S10.002	S54	15 Winter	100	+40%	100/15 Summer		
S10.003	S55	15 Winter	100	+40%	100/15 Summer		
S7.011	S56	15 Winter	100	+40%	100/15 Summer	100/15 Winter	
S7.012	S57	15 Winter	100	+40%	100/15 Summer		
S12.000	S58	15 Winter	100	+40%	100/15 Summer		
S12.001	S59	15 Winter	100	+40%	100/15 Summer		
S13.000	S60	15 Winter	100	+40%	100/15 Summer		
S14.000	S61	15 Winter	100	+40%	100/15 Summer	100/15 Summer	
S12.002	S62	15 Winter	100	+40%	100/15 Summer		
S12.003	S63	15 Winter	100	+40%	100/15 Summer		
S12.004	S64	15 Winter	100	+40%	100/15 Summer		
S12.005	S65	30 Winter	100	+40%	100/15 Summer	100/15 Summer	
S12.006	S66	30 Winter	100	+40%	100/15 Summer	100/15 Summer	
S12.007	S67	30 Winter	100	+40%	100/15 Summer	100/15 Summer	
S12.008	S68	15 Winter	100	+40%	100/15 Summer		
S15.000	SPP4	60 Winter	100	+40%	100/15 Summer		
S15.001	SHBRAKE PP4	120 Winter	100	+40%	100/15 Summer		
S15.002	S71	15 Winter	100	+40%	100/15 Summer		
S15.003	S72	30 Winter	100	+40%	100/15 Summer	100/15 Summer	
S15.004	S73	15 Winter	100	+40%	100/15 Summer	100/15 Summer	
S15.005	S74	15 Winter	100	+40%	100/15 Summer		
S15.006	S75	15 Winter	100	+40%	100/15 Summer		
S15.007	S76	15 Winter	100	+40%	100/15 Summer		
S12.009	S77	15 Winter	100	+40%	100/15 Summer		
S7.013	S78	15 Winter	100	+40%	100/15 Summer		
S7.014	S79	15 Winter	100	+40%	100/15 Summer		
S7.015	S80	480 Winter	100	+40%	100/15 Summer		
S7.016	SBASIN K	480 Winter	100	+40%	100/30 Winter		
S7.017	SHBRAKE BASIN K	960 Winter	100	+40%			

PN	US/MH Name	Overflow Act.	Water Level (m)	Surcharged Depth (m)	Flooded Volume (m³)	Flow / Overflow Cap.	Half Drain Time (mins)
S11.001	S51		22.008	1.349	0.000	0.57	
S11.002	S52		21.980	1.488	0.000	0.70	
S10.001	S53		21.953	1.538	0.000	1.18	
S10.002	S54		21.823	1.518	0.000	1.76	
S10.003	S55		21.603	1.381	0.000	0.78	
S7.011	S56		21.414	1.558	0.067	1.68	
S7.012	S57		21.120	1.373	0.000	1.27	
S12.000	S58		22.275	0.342	0.000	0.58	
S12.001	S59		22.241	0.419	0.000	0.47	
S13.000	S60		22.376	0.749	0.000	0.62	
S14.000	S61		22.050	1.202	2.134	0.38	

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<u>Summary of Critical Results by Maximum Level (Rank 1) for Storm</u>					
PN	US/MH Name	Pipe Flow (l/s)	Status	Level Exceeded	
S15.004		S73 43.7	FLOOD		4
S15.005		S74 59.0	FLOOD RISK		
S15.006		S75 69.3	FLOOD RISK		
S15.007		S76 63.9	FLOOD RISK		
S12.009		S77 167.6	FLOOD RISK		
S7.013		S78 297.6	SURCHARGED		
S7.014		S79 285.4	SURCHARGED		
S7.015		S80 92.6	SURCHARGED		
S7.016	SBASIN K	13.6	SURCHARGED		
S7.017	SHBRAKE BASIN K	13.6	OK		
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STORM SEWER DESIGN by the Modified Rational Method

Design Criteria for Storm

Pipe Sizes STANDARD Manhole Sizes STANDARD

FSR Rainfall Model - England and Wales

Return Period (years)	100	PIMP (%)	100
M5-60 (mm)	19.900	Add Flow / Climate Change (%)	0
Ratio R	0.350	Minimum Backdrop Height (m)	0.200
Maximum Rainfall (mm/hr)	50	Maximum Backdrop Height (m)	1.500
Maximum Time of Concentration (mins)	30	Min Design Depth for Optimisation (m)	1.200
Foul Sewage (l/s/ha)	0.000	Min Vel for Auto Design only (m/s)	1.00
Volumetric Runoff Coeff.	0.750	Min Slope for Optimisation (1:X)	500

Designed with Level Soffits

Time Area Diagram for Storm

Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)
0-4	0.018	4-8	0.713	8-12	1.714	12-16	0.248

Total Area Contributing (ha) = 2.694

Total Pipe Volume (m³) = 274.840

Network Design Table for Storm

PN	Length (m)	Fall (m)	Slope (1:X)	I.Area (ha)	T.E. (mins)	Base Flow (l/s)	k (mm)	HYD SECT	DIA (mm)	Section Type	Auto Design
S16.000	22.220	0.333	66.7	0.024	4.00	0.0	0.600	o	150	Pipe/Conduit	
S17.000	21.574	0.809	26.7	0.041	4.00	0.0	0.600	o	150	Pipe/Conduit	
S17.001	26.648	0.919	29.0	0.053	0.00	0.0	0.600	o	225	Pipe/Conduit	
S16.001	26.563	0.395	67.2	0.049	0.00	0.0	0.600	o	300	Pipe/Conduit	
S16.002	29.160	0.384	75.9	0.047	0.00	0.0	0.600	o	300	Pipe/Conduit	

Network Results Table

PN	Rain (mm/hr)	T.C. (mins)	US/IL (m)	E I.Area (ha)	E Base Flow (l/s)	Foul (l/s)	Add Flow (l/s)	Vel (m/s)	Cap (l/s)	Flow (l/s)
S16.000	50.00	4.30	25.130	0.024	0.0	0.0	0.0	1.23	21.8	3.2
S17.000	50.00	4.18	26.525	0.041	0.0	0.0	0.0	1.96	34.6	5.6
S17.001	50.00	4.37	25.641	0.094	0.0	0.0	0.0	2.44	97.0	12.7
S16.001	50.00	4.60	24.647	0.167	0.0	0.0	0.0	1.92	135.7	22.6
S16.002	50.00	4.87	24.252	0.214	0.0	0.0	0.0	1.81	127.7	29.0

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<u>Network Design Table for Storm</u>											
PN	Length (m)	Fall (m)	Slope (1:X)	I.Area (ha)	T.E. (mins)	Base Flow (l/s)	k (mm)	HYD SECT	DIA (mm)	Section Type	Auto Design
S18.000	22.692	0.795	28.5	0.045	4.00	0.0	0.600	o	150	Pipe/Conduit	
S18.001	22.747	0.820	27.7	0.077	0.00	0.0	0.600	o	225	Pipe/Conduit	
S19.000	17.312	0.345	50.2	0.011	4.00	0.0	0.600	o	150	Pipe/Conduit	
S19.001	10.325	0.195	52.9	0.028	0.00	0.0	0.600	o	225	Pipe/Conduit	
S19.002	13.321	0.253	52.7	0.113	0.00	0.0	0.600	o	300	Pipe/Conduit	
S19.003	20.903	0.379	55.2	0.008	0.00	0.0	0.600	o	300	Pipe/Conduit	
S16.003	23.198	0.841	27.6	0.104	0.00	0.0	0.600	o	450	Pipe/Conduit	
S16.004	6.114	0.196	31.2	0.012	0.00	0.0	0.600	o	525	Pipe/Conduit	
S20.000	36.476	0.510	71.5	0.014	4.00	0.0	0.600	o	150	Pipe/Conduit	
S20.001	14.010	0.093	150.6	0.034	0.00	0.0	0.600	o	150	Pipe/Conduit	
S16.005	6.437	0.118	54.6	0.011	0.00	0.0	0.600	o	525	Pipe/Conduit	
S16.006	10.922	0.128	85.3	0.000	0.00	0.0	0.600	o	525	Pipe/Conduit	
S16.007	8.616	0.082	105.1	0.030	0.00	0.0	0.600	o	525	Pipe/Conduit	
S16.008	24.413	0.251	97.3	0.000	0.00	0.0	0.600	o	525	Pipe/Conduit	
S16.009	25.396	0.250	101.6	0.127	0.00	0.0	0.600	o	525	Pipe/Conduit	
S16.010	13.755	0.103	133.5	0.055	0.00	0.0	0.600	o	600	Pipe/Conduit	
S16.011	18.458	0.100	184.6	0.078	0.00	0.0	0.600	o	600	Pipe/Conduit	
<u>Network Results Table</u>											
PN	Rain (mm/hr)	T.C. (mins)	US/IL (m)	Σ I.Area (ha)	Σ Base Flow (l/s)	Foul (l/s)	Add Flow (l/s)	Vel (m/s)	Cap (l/s)	Flow (l/s)	
S18.000	50.00	4.20	25.633	0.045	0.0	0.0	0.0	1.89	33.4	6.1	
S18.001	50.00	4.35	24.763	0.122	0.0	0.0	0.0	2.49	99.2	16.5	
S19.000	50.00	4.20	25.190	0.011	0.0	0.0	0.0	1.42	25.2	1.5	
S19.001	50.00	4.30	24.770	0.039	0.0	0.0	0.0	1.80	71.6	5.3	
S19.002	50.00	4.40	24.500	0.152	0.0	0.0	0.0	2.17	153.5	20.6	
S19.003	50.00	4.56	24.247	0.160	0.0	0.0	0.0	2.12	150.0	21.7	
S16.003	50.00	4.97	23.718	0.600	0.0	0.0	0.0	3.88	617.5	81.2	
S16.004	50.00	4.99	22.802	0.612	0.0	0.0	0.0	4.02	870.4	82.9	
S20.000	50.00	4.51	23.584	0.014	0.0	0.0	0.0	1.19	21.0	1.9	
S20.001	50.00	4.80	23.074	0.048	0.0	0.0	0.0	0.82	14.4	6.5	
S16.005	50.00	5.03	22.606	0.671	0.0	0.0	0.0	3.04	657.5	90.9	
S16.006	50.00	5.10	22.488	0.671	0.0	0.0	0.0	2.43	525.2	90.9	
S16.007	50.00	5.17	22.360	0.701	0.0	0.0	0.0	2.18	473.0	94.9	
S16.008	50.00	5.35	22.278	0.701	0.0	0.0	0.0	2.27	491.7	94.9	
S16.009	50.00	5.54	22.027	0.828	0.0	0.0	0.0	2.22	481.1	112.1	
S16.010	50.00	5.64	21.702	0.883	0.0	0.0	0.0	2.11	595.4	119.6	
S16.011	50.00	5.82	21.599	0.961	0.0	0.0	0.0	1.79	505.9	130.1	
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Network Design Table for Storm

PN	Length (m)	Fall (m)	Slope (1:X)	I.Area (ha)	T.E. (mins)	Base Flow (l/s)	k (mm)	HYD SECT	DIA (mm)	Section Type	Auto Design
S16.012	28.539	0.357	79.9	0.091	0.00	0.0	0.600	o	600	Pipe/Conduit	
S16.013	29.655	0.053	556.0	0.045	0.00	0.0	0.600	o	600	Pipe/Conduit	
S16.014	24.556	0.044	556.0	0.045	0.00	0.0	0.600	o	600	Pipe/Conduit	
S16.015	23.901	0.043	556.0	0.000	0.00	0.0	0.600	o	600	Pipe/Conduit	
S16.016	26.254	0.089	295.0	0.051	0.00	0.0	0.600	o	600	Pipe/Conduit	
S21.000	4.798	0.604	7.9	0.016	4.00	0.0	0.600	o	150	Pipe/Conduit	
S21.001	6.858	0.125	54.9	0.058	0.00	0.0	0.600	o	150	Pipe/Conduit	
S21.002	19.437	0.268	72.5	0.024	0.00	0.0	0.600	o	225	Pipe/Conduit	
S21.003	28.496	0.300	95.0	0.082	0.00	0.0	0.600	o	225	Pipe/Conduit	
S21.004	17.799	0.200	89.0	0.032	0.00	0.0	0.600	o	225	Pipe/Conduit	
S21.005	15.802	0.066	238.0	0.038	0.00	0.0	0.600	o	300	Pipe/Conduit	
S21.006	14.733	0.062	238.0	0.041	0.00	0.0	0.600	o	300	Pipe/Conduit	
S21.007	15.080	0.063	238.0	0.034	0.00	0.0	0.600	o	300	Pipe/Conduit	
S21.008	26.887	0.113	237.9	0.024	0.00	0.0	0.600	o	300	Pipe/Conduit	
S22.000	7.673	0.676	11.4	0.019	4.00	0.0	0.600	o	150	Pipe/Conduit	
S22.001	7.875	0.971	8.1	0.000	0.00	0.0	0.600	o	150	Pipe/Conduit	
S21.009	21.412	0.090	237.9	0.061	0.00	0.0	0.600	o	300	Pipe/Conduit	
S23.000	31.295	0.209	150.0	0.022	4.00	0.0	0.600	o	150	Pipe/Conduit	

Network Results Table

PN	Rain (mm/hr)	T.C. (mins)	US/IL (m)	Σ I.Area (ha)	Σ Base Flow (l/s)	Foul (l/s)	Add Flow (l/s)	Vel (m/s)	Cap (l/s)	Flow (l/s)
S16.012	50.00	5.99	21.499	1.052	0.0	0.0	0.0	2.73	770.6	142.5
S16.013	50.00	6.47	21.142	1.097	0.0	0.0	0.0	1.03	290.0	148.5
S16.014	50.00	6.87	21.089	1.142	0.0	0.0	0.0	1.03	290.0	154.6
S16.015	50.00	7.26	21.044	1.142	0.0	0.0	0.0	1.03	290.0	154.6
S16.016	50.00	7.57	21.002	1.193	0.0	0.0	0.0	1.41	399.4	161.5
S21.000	50.00	4.02	23.754	0.016	0.0	0.0	0.0	3.60	63.6	2.2
S21.001	50.00	4.11	23.150	0.074	0.0	0.0	0.0	1.36	24.0	10.0
S21.002	50.00	4.32	22.950	0.098	0.0	0.0	0.0	1.54	61.1	13.3
S21.003	50.00	4.67	22.682	0.180	0.0	0.0	0.0	1.34	53.4	24.4
S21.004	50.00	4.88	22.382	0.212	0.0	0.0	0.0	1.39	55.1	28.7
S21.005	50.00	5.14	22.107	0.250	0.0	0.0	0.0	1.01	71.7	33.9
S21.006	50.00	5.39	22.041	0.291	0.0	0.0	0.0	1.01	71.7	39.4
S21.007	50.00	5.63	21.979	0.325	0.0	0.0	0.0	1.01	71.7	44.0
S21.008	50.00	6.08	21.915	0.349	0.0	0.0	0.0	1.01	71.7	47.3
S22.000	50.00	4.04	23.599	0.019	0.0	0.0	0.0	3.01	53.1	2.6
S22.001	50.00	4.08	22.923	0.019	0.0	0.0	0.0	3.56	62.9	2.6
S21.009	50.00	6.43	21.802	0.429	0.0	0.0	0.0	1.01	71.7	58.1
S23.000	50.00	4.64	23.661	0.022	0.0	0.0	0.0	0.82	14.5	3.0

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Manhole Schedules for Storm

MH Name	MH CL (m)	MH Depth (m)	MH Connection	MH Diam., L*W (mm)	PN	Pipe Out Invert Level (m)	Diameter (mm)	PN	Pipes In Invert Level (m)	Diameter (mm)	Base
S83	26.480	1.350	Open Manhole	1200	S16.000	25.130	150				
S84	27.875	1.350	Open Manhole	1200	S17.000	26.525	150				
S85	27.066	1.425	Open Manhole	1200	S17.001	25.641	225	S17.000	25.716	150	
S86	26.147	1.500	Open Manhole	1200	S16.001	24.647	300	S16.000	24.797	150	
								S17.001	24.722	225	
S87	25.752	1.500	Open Manhole	1200	S16.002	24.252	300	S16.001	24.252	300	
S88	26.983	1.350	Open Manhole	1200	S18.000	25.633	150				
S89	26.188	1.425	Open Manhole	1200	S18.001	24.763	225	S18.000	24.838	150	
S90	26.540	1.350	Open Manhole	1200	S19.000	25.190	150				
S91	26.195	1.425	Open Manhole	1200	S19.001	24.770	225	S19.000	24.845	150	
S92	26.000	1.500	Open Manhole	1200	S19.002	24.500	300	S19.001	24.575	225	
S93	25.747	1.500	Open Manhole	1200	S19.003	24.247	300	S19.002	24.247	300	
S94	25.368	1.650	Open Manhole	1350	S16.003	23.718	450	S16.002	23.868	300	
								S18.001	23.943	225	
								S19.003	23.868	300	
S95	24.527	1.725	Open Manhole	1500	S16.004	22.802	525	S16.003	22.877	450	
S96	24.934	1.350	Open Manhole	1200	S20.000	23.584	150				
S97	24.424	1.350	Open Manhole	1200	S20.001	23.074	150	S20.000	23.074	150	
S98	24.346	1.740	Open Manhole	1500	S16.005	22.606	525	S16.004	22.606	525	
								S20.001	22.981	150	
S99	24.213	1.725	Open Manhole	1500	S16.006	22.488	525	S16.005	22.488	525	
S100	24.085	1.725	Open Manhole	1500	S16.007	22.360	525	S16.006	22.360	525	
S101	24.003	1.725	Open Manhole	1500	S16.008	22.278	525	S16.007	22.278	525	
S102	23.752	1.725	Open Manhole	1500	S16.009	22.027	525	S16.008	22.027	525	
S103	23.502	1.800	Open Manhole	1500	S16.010	21.702	600	S16.009	21.777	525	
S104	23.400	1.801	Open Manhole	1500	S16.011	21.599	600	S16.010	21.599	600	
S105	23.300	1.801	Open Manhole	1500	S16.012	21.499	600	S16.011	21.499	600	
S106	23.540	2.398	Open Manhole	1500	S16.013	21.142	600	S16.012	21.142	600	
S107	23.900	2.811	Open Manhole	1500	S16.014	21.089	600	S16.013	21.089	600	
S108	24.100	3.056	Open Manhole	1500	S16.015	21.044	600	S16.014	21.044	600	
S109	23.960	2.958	Open Manhole	1500	S16.016	21.002	600	S16.015	21.002	600	
SPP5	24.554	0.800	Open Manhole	1200	S21.000	23.754	150				
SHBRAKE PP5	24.500	1.350	Open Manhole	1200	S21.001	23.150	150	S21.000	23.150	150	
S112	24.375	1.425	Open Manhole	1200	S21.002	22.950	225	S21.001	23.025	150	
S113	24.107	1.425	Open Manhole	1200	S21.003	22.682	225	S21.002	22.682	225	
S114	23.807	1.425	Open Manhole	1200	S21.004	22.382	225	S21.003	22.382	225	
S115	23.607	1.500	Open Manhole	1200	S21.005	22.107	300	S21.004	22.182	225	
S116	23.593	1.553	Open Manhole	1200	S21.006	22.041	300	S21.005	22.041	300	

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Manhole Schedules for Storm

MH Name	MH CL (m)	MH Depth (m)	MH Connection	MH Diam., L*W (mm)	PN	Pipe Out Invert Level (m)	Diameter (mm)	PN	Pipes In Invert Level (m)	Diameter (mm)	Bar
S117	23.716	1.737	Open Manhole	1200	S21.007	21.979	300	S21.006	21.979	300	
S118	23.862	1.947	Open Manhole	1200	S21.008	21.915	300	S21.007	21.915	300	
SPP6	24.399	0.800	Open Manhole	1200	S22.000	23.599	150				
SHBRAKE PP6	24.273	1.350	Open Manhole	1200	S22.001	22.923	150	S22.000	22.923	150	
S121	24.134	2.332	Open Manhole	1200	S21.009	21.802	300	S21.008	21.802	300	
								S22.001	21.952	150	
S122	25.011	1.350	Open Manhole	1200	S23.000	23.661	150				
S123	25.196	1.744	Open Manhole	1200	S23.001	23.452	150	S23.000	23.452	150	
S124	25.035	1.625	Open Manhole	1200	S23.002	23.410	150	S23.001	23.410	150	
S125	24.700	3.138	Open Manhole	1350	S21.010	21.562	450	S21.009	21.712	300	
								S23.002	21.862	150	
S126	23.700	2.238	Open Manhole	1350	S21.011	21.462	450	S21.010	21.462	450	
S127	23.470	2.143	Open Manhole	1500	S21.012	21.327	525	S21.011	21.402	450	
S128	23.250	2.007	Open Manhole	1500	S21.013	21.243	525	S21.012	21.243	525	
S129	23.500	2.325	Open Manhole	1500	S21.014	21.175	525	S21.013	21.175	525	
S130	23.600	2.687	Open Manhole	1500	S16.017	20.913	600	S16.016	20.913	600	
								S21.014	20.988	525	
S131	23.200	2.347	Open Manhole	1500	S16.018	20.853	600	S16.017	20.853	600	
S132	22.600	1.961	Open Manhole	1500	S16.019	20.639	600	S16.018	20.639	600	
S133	22.100	1.961	Open Manhole	1500	S16.020	20.139	600	S16.019	20.139	600	
SPP7	22.625	0.800	Open Manhole	1200	S24.000	21.825	150				
SHBRAKE PP7	22.878	1.350	Open Manhole	1200	S24.001	21.528	150	S24.000	21.528	150	
S136	22.756	1.350	Open Manhole	1200	S24.002	21.406	150	S24.001	21.406	150	
S137	22.305	1.425	Open Manhole	1200	S24.003	20.880	225	S24.002	20.955	150	
S138	21.804	1.425	Open Manhole	1200	S24.004	20.379	225	S24.003	20.379	225	
SPP8	21.536	0.800	Open Manhole	1200	S25.000	20.736	150				
SHBRAKE PP8	21.790	1.308	Open Manhole	1200	S25.001	20.482	225	S25.000	20.557	150	
S141	21.518	1.197	Open Manhole	1200	S25.002	20.321	225	S25.001	20.321	225	
S142	21.537	1.372	Open Manhole	1200	S24.005	20.165	300	S24.004	20.240	225	
								S25.002	20.240	225	
S143	21.713	1.790	Open Manhole	1350	S24.006	19.923	450	S24.005	20.073	300	
S144	21.900	2.334	Open Manhole	1500	S16.021	19.566	675	S16.020	19.641	600	
								S24.006	19.876	450	
S145	22.040	2.536	Open Manhole	1500	S16.022	19.504	675	S16.021	19.504	675	
S146	22.230	2.758	Open Manhole	1500	S16.023	19.472	675	S16.022	19.472	675	
S147	22.300	2.864	Open Manhole	1500	S16.024	19.436	675	S16.023	19.436	675	
S148	22.150	2.758	Open Manhole	1500	S16.025	19.392	675	S16.024	19.392	675	
S149	21.900	2.544	Open Manhole	1500	S16.026	19.356	675	S16.025	19.356	675	

Manhole Schedules for Storm

MH Name	Manhole Easting (m)	Manhole Northing (m)	Intersection Easting (m)	Intersection Northing (m)	Manhole Access	Layout (North)
SHBRAKE PP8	448871.111	116412.787	448871.111	116412.787	Required	
S141	448848.765	116421.977	448848.765	116421.977	Required	
S142	448837.275	116418.068	448837.275	116418.068	Required	
S143	448816.824	116410.547	448816.824	116410.547	Required	
S144	448799.690	116404.345	448799.690	116404.345	Required	
S145	448791.513	116437.908	448791.513	116437.908	Required	
S146	448785.082	116454.907	448785.082	116454.907	Required	
S147	448776.845	116472.765	448776.845	116472.765	Required	
S148	448775.997	116497.240	448775.997	116497.240	Required	
S149	448777.019	116517.366	448777.019	116517.366	Required	
SBASIN L	448721.521	116591.558	448721.521	116591.558	Required	
SHBRAKE BASIN L	448668.896	116664.718	448668.896	116664.718	Required	
S	448693.220	116696.311			No Entry	
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PIPELINE SCHEDULES for Storm

Upstream Manhole

PN	Hyd Sect	Diam (mm)	MH Name	C.Level (m)	I.Level (m)	D.Depth (m)	MH Connection	MH DIAM., L*W (mm)
S16.000	o	150	S83	26.480	25.130	1.200	Open Manhole	1200
S17.000	o	150	S84	27.875	26.525	1.200	Open Manhole	1200
S17.001	o	225	S85	27.066	25.641	1.200	Open Manhole	1200
S16.001	o	300	S86	26.147	24.647	1.200	Open Manhole	1200
S16.002	o	300	S87	25.752	24.252	1.200	Open Manhole	1200
S18.000	o	150	S88	26.983	25.633	1.200	Open Manhole	1200
S18.001	o	225	S89	26.188	24.763	1.200	Open Manhole	1200
S19.000	o	150	S90	26.540	25.190	1.200	Open Manhole	1200
S19.001	o	225	S91	26.195	24.770	1.200	Open Manhole	1200
S19.002	o	300	S92	26.000	24.500	1.200	Open Manhole	1200
S19.003	o	300	S93	25.747	24.247	1.200	Open Manhole	1200
S16.003	o	450	S94	25.368	23.718	1.200	Open Manhole	1350
S16.004	o	525	S95	24.527	22.802	1.200	Open Manhole	1500
S20.000	o	150	S96	24.934	23.584	1.200	Open Manhole	1200
S20.001	o	150	S97	24.424	23.074	1.200	Open Manhole	1200

Downstream Manhole

PN	Length (m)	Slope (1:X)	MH Name	C.Level (m)	I.Level (m)	D.Depth (m)	MH Connection	MH DIAM., L*W (mm)
S16.000	22.220	66.7	S86	26.147	24.797	1.200	Open Manhole	1200
S17.000	21.574	26.7	S85	27.066	25.716	1.200	Open Manhole	1200
S17.001	26.648	29.0	S86	26.147	24.722	1.200	Open Manhole	1200
S16.001	26.563	67.2	S87	25.752	24.252	1.200	Open Manhole	1200
S16.002	29.160	75.9	S94	25.368	23.868	1.200	Open Manhole	1350
S18.000	22.692	28.5	S89	26.188	24.838	1.200	Open Manhole	1200
S18.001	22.747	27.7	S94	25.368	23.943	1.200	Open Manhole	1350
S19.000	17.312	50.2	S91	26.195	24.845	1.200	Open Manhole	1200
S19.001	10.325	52.9	S92	26.000	24.575	1.200	Open Manhole	1200
S19.002	13.321	52.7	S93	25.747	24.247	1.200	Open Manhole	1200
S19.003	20.903	55.2	S94	25.368	23.868	1.200	Open Manhole	1350
S16.003	23.198	27.6	S95	24.527	22.877	1.200	Open Manhole	1500
S16.004	6.114	31.2	S98	24.346	22.606	1.215	Open Manhole	1500
S20.000	36.476	71.5	S97	24.424	23.074	1.200	Open Manhole	1200
S20.001	14.010	150.6	S98	24.346	22.981	1.215	Open Manhole	1500

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Upstream Manhole									
PN	Hyd Sect	Diam (mm)	MH Name	C.Level (m)	I.Level (m)	D.Depth (m)	MH Connection	MH DIAM., (mm)	L*W
S16.005	o	525	S98	24.346	22.606	1.215	Open Manhole		1500
S16.006	o	525	S99	24.213	22.488	1.200	Open Manhole		1500
S16.007	o	525	S100	24.085	22.360	1.200	Open Manhole		1500
S16.008	o	525	S101	24.003	22.278	1.200	Open Manhole		1500
S16.009	o	525	S102	23.752	22.027	1.200	Open Manhole		1500
S16.010	o	600	S103	23.502	21.702	1.200	Open Manhole		1500
S16.011	o	600	S104	23.400	21.599	1.201	Open Manhole		1500
S16.012	o	600	S105	23.300	21.499	1.201	Open Manhole		1500
S16.013	o	600	S106	23.540	21.142	1.798	Open Manhole		1500
S16.014	o	600	S107	23.900	21.089	2.211	Open Manhole		1500
S16.015	o	600	S108	24.100	21.044	2.456	Open Manhole		1500
S16.016	o	600	S109	23.960	21.002	2.358	Open Manhole		1500
S21.000	o	150	SPP5	24.554	23.754	0.650	Open Manhole		1200
S21.001	o	150	SHBRAKE PP5	24.500	23.150	1.200	Open Manhole		1200
S21.002	o	225	S112	24.375	22.950	1.200	Open Manhole		1200
S21.003	o	225	S113	24.107	22.682	1.200	Open Manhole		1200
S21.004	o	225	S114	23.807	22.382	1.200	Open Manhole		1200
S21.005	o	300	S115	23.607	22.107	1.200	Open Manhole		1200
S21.006	o	300	S116	23.593	22.041	1.253	Open Manhole		1200
Downstream Manhole									
PN	Length (m)	Slope (1:X)	MH Name	C.Level (m)	I.Level (m)	D.Depth (m)	MH Connection	MH DIAM., (mm)	L*W
S16.005	6.437	54.6	S99	24.213	22.488	1.200	Open Manhole		1500
S16.006	10.922	85.3	S100	24.085	22.360	1.200	Open Manhole		1500
S16.007	8.616	105.1	S101	24.003	22.278	1.200	Open Manhole		1500
S16.008	24.413	97.3	S102	23.752	22.027	1.200	Open Manhole		1500
S16.009	25.396	101.6	S103	23.502	21.777	1.200	Open Manhole		1500
S16.010	13.755	133.5	S104	23.400	21.599	1.201	Open Manhole		1500
S16.011	18.458	184.6	S105	23.300	21.499	1.201	Open Manhole		1500
S16.012	28.539	79.9	S106	23.540	21.142	1.798	Open Manhole		1500
S16.013	29.655	556.0	S107	23.900	21.089	2.211	Open Manhole		1500
S16.014	24.556	556.0	S108	24.100	21.044	2.456	Open Manhole		1500
S16.015	23.901	556.0	S109	23.960	21.002	2.358	Open Manhole		1500
S16.016	26.254	295.0	S130	23.600	20.913	2.087	Open Manhole		1500
S21.000	4.798	7.9	SHBRAKE PP5	24.500	23.150	1.200	Open Manhole		1200
S21.001	6.858	54.9	S112	24.375	23.025	1.200	Open Manhole		1200
S21.002	19.437	72.5	S113	24.107	22.682	1.200	Open Manhole		1200
S21.003	28.496	95.0	S114	23.807	22.382	1.200	Open Manhole		1200
S21.004	17.799	89.0	S115	23.607	22.182	1.200	Open Manhole		1200
S21.005	15.802	238.0	S116	23.593	22.041	1.253	Open Manhole		1200
S21.006	14.733	238.0	S117	23.716	21.979	1.437	Open Manhole		1200
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PIPELINE SCHEDULES for Storm

Upstream Manhole

PN	Hyd Sect	Diam (mm)	MH Name	C.Level (m)	I.Level (m)	D.Depth (m)	MH Connection	MH DIAM., L*W (mm)
S21.007	o	300	S117	23.716	21.979	1.437	Open Manhole	1200
S21.008	o	300	S118	23.862	21.915	1.647	Open Manhole	1200
S22.000	o	150	SPP6	24.399	23.599	0.650	Open Manhole	1200
S22.001	o	150	SHBRAKE PP6	24.273	22.923	1.200	Open Manhole	1200
S21.009	o	300	S121	24.134	21.802	2.032	Open Manhole	1200
S23.000	o	150	S122	25.011	23.661	1.200	Open Manhole	1200
S23.001	o	150	S123	25.196	23.452	1.594	Open Manhole	1200
S23.002	o	150	S124	25.035	23.410	1.475	Open Manhole	1200
S21.010	o	450	S125	24.700	21.562	2.688	Open Manhole	1350
S21.011	o	450	S126	23.700	21.462	1.788	Open Manhole	1350
S21.012	o	525	S127	23.470	21.327	1.618	Open Manhole	1500
S21.013	o	525	S128	23.250	21.243	1.482	Open Manhole	1500
S21.014	o	525	S129	23.500	21.175	1.800	Open Manhole	1500
S16.017	o	600	S130	23.600	20.913	2.087	Open Manhole	1500
S16.018	o	600	S131	23.200	20.853	1.747	Open Manhole	1500
S16.019	o	600	S132	22.600	20.639	1.361	Open Manhole	1500

Downstream Manhole

PN	Length (m)	Slope (1:X)	MH Name	C.Level (m)	I.Level (m)	D.Depth (m)	MH Connection	MH DIAM., L*W (mm)
S21.007	15.080	238.0	S118	23.862	21.915	1.647	Open Manhole	1200
S21.008	26.887	237.9	S121	24.134	21.802	2.031	Open Manhole	1200
S22.000	7.673	11.4	SHBRAKE PP6	24.273	22.923	1.200	Open Manhole	1200
S22.001	7.875	8.1	S121	24.134	21.952	2.032	Open Manhole	1200
S21.009	21.412	237.9	S125	24.700	21.712	2.688	Open Manhole	1350
S23.000	31.295	150.0	S123	25.196	23.452	1.594	Open Manhole	1200
S23.001	6.332	150.0	S124	25.035	23.410	1.475	Open Manhole	1200
S23.002	12.179	7.9	S125	24.700	21.862	2.688	Open Manhole	1350
S21.010	38.483	384.8	S126	23.700	21.462	1.788	Open Manhole	1350
S21.011	23.055	384.2	S127	23.470	21.402	1.618	Open Manhole	1500
S21.012	38.011	452.5	S128	23.250	21.243	1.482	Open Manhole	1500
S21.013	31.029	456.3	S129	23.500	21.175	1.800	Open Manhole	1500
S21.014	12.375	66.2	S130	23.600	20.988	2.087	Open Manhole	1500
S16.017	32.829	556.0	S131	23.200	20.853	1.747	Open Manhole	1500
S16.018	35.912	167.8	S132	22.600	20.639	1.361	Open Manhole	1500
S16.019	32.324	64.6	S133	22.100	20.139	1.361	Open Manhole	1500

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PIPELINE SCHEDULES for Storm									
Upstream Manhole									
PN	Hyd Sect	Diam (mm)	MH Name	C.Level (m)	I.Level (m)	D.Depth (m)	MH Connection	MH DIAM., (mm)	L*W
S16.020	o	600	S133	22.100	20.139	1.361	Open Manhole		1500
S24.000	o	150	SPP7	22.625	21.825	0.650	Open Manhole		1200
S24.001	o	150	SHBRAKE PP7	22.878	21.528	1.200	Open Manhole		1200
S24.002	o	150	S136	22.756	21.406	1.200	Open Manhole		1200
S24.003	o	225	S137	22.305	20.880	1.200	Open Manhole		1200
S24.004	o	225	S138	21.804	20.379	1.200	Open Manhole		1200
S25.000	o	150	SPP8	21.536	20.736	0.650	Open Manhole		1200
S25.001	o	225	SHBRAKE PP8	21.790	20.482	1.083	Open Manhole		1200
S25.002	o	225	S141	21.518	20.321	0.972	Open Manhole		1200
S24.005	o	300	S142	21.537	20.165	1.072	Open Manhole		1200
S24.006	o	450	S143	21.713	19.923	1.340	Open Manhole		1350
S16.021	o	675	S144	21.900	19.566	1.659	Open Manhole		1500
S16.022	o	675	S145	22.040	19.504	1.861	Open Manhole		1500
S16.023	o	675	S146	22.230	19.472	2.083	Open Manhole		1500
S16.024	o	675	S147	22.300	19.436	2.189	Open Manhole		1500
S16.025	o	675	S148	22.150	19.392	2.083	Open Manhole		1500
S16.026	o	675	S149	21.900	19.356	1.869	Open Manhole		1500
Downstream Manhole									
PN	Length (m)	Slope (1:X)	MH Name	C.Level (m)	I.Level (m)	D.Depth (m)	MH Connection	MH DIAM., (mm)	L*W
S16.020	24.523	49.2	S144	21.900	19.641	1.659	Open Manhole		1500
S24.000	6.160	20.7	SHBRAKE PP7	22.878	21.528	1.200	Open Manhole		1200
S24.001	7.988	65.5	S136	22.756	21.406	1.200	Open Manhole		1200
S24.002	28.632	63.5	S137	22.305	20.955	1.200	Open Manhole		1200
S24.003	30.090	60.1	S138	21.804	20.379	1.200	Open Manhole		1200
S24.004	12.984	93.4	S142	21.537	20.240	1.072	Open Manhole		1200
S25.000	6.940	38.8	SHBRAKE PP8	21.790	20.557	1.083	Open Manhole		1200
S25.001	24.162	150.1	S141	21.518	20.321	0.972	Open Manhole		1200
S25.002	12.137	149.8	S142	21.537	20.240	1.072	Open Manhole		1200
S24.005	21.790	238.0	S143	21.713	20.073	1.340	Open Manhole		1350
S24.006	18.221	385.0	S144	21.900	19.876	1.574	Open Manhole		1500
S16.021	34.545	556.0	S145	22.040	19.504	1.861	Open Manhole		1500
S16.022	18.175	556.0	S146	22.230	19.472	2.083	Open Manhole		1500
S16.023	19.666	556.0	S147	22.300	19.436	2.189	Open Manhole		1500
S16.024	24.490	556.0	S148	22.150	19.392	2.083	Open Manhole		1500
S16.025	20.152	556.0	S149	21.900	19.356	1.869	Open Manhole		1500
S16.026	92.652	593.9	SBASIN L	20.100	19.200	0.225	Open Manhole		1500

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PIPELINE SCHEDULES for Storm

Upstream Manhole

PN	Hyd Sect	Diam (mm)	MH Name	C.Level (m)	I.Level (m)	D.Depth (m)	MH Connection	MH DIAM., L*W (mm)
S16.027	o	675	SBASIN L	20.100	19.200	0.225	Open Manhole	1500
S16.028	o	450	SHBRAKE BASIN L	20.000	18.949	0.601	Open Manhole	1500

Downstream Manhole

PN	Length (m)	Slope (1:X)	MH Name	C.Level (m)	I.Level (m)	D.Depth (m)	MH Connection	MH DIAM., L*W (mm)
S16.027	90.121	359.0	SHBRAKE BASIN L	20.000	18.949	0.376	Open Manhole	1500
S16.028	39.872	20.4	S	17.500	16.999	0.051	Open Manhole	0

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<u>Area Summary for Storm</u>						
Pipe Number	PIMP Type	PIMP Name	PIMP (%)	Gross Area (ha)	Imp. Area (ha)	Pipe Total (ha)
16.000	-	-	100	0.024	0.024	0.024
17.000	-	-	100	0.041	0.041	0.041
17.001	-	-	100	0.053	0.053	0.053
16.001	-	-	100	0.049	0.049	0.049
16.002	-	-	100	0.047	0.047	0.047
18.000	-	-	100	0.045	0.045	0.045
18.001	-	-	100	0.077	0.077	0.077
19.000	-	-	100	0.011	0.011	0.011
19.001	-	-	100	0.028	0.028	0.028
19.002	-	-	100	0.113	0.113	0.113
19.003	-	-	100	0.008	0.008	0.008
16.003	-	-	100	0.104	0.104	0.104
16.004	-	-	100	0.012	0.012	0.012
20.000	-	-	100	0.014	0.014	0.014
20.001	-	-	100	0.034	0.034	0.034
16.005	-	-	100	0.011	0.011	0.011
16.006	-	-	100	0.000	0.000	0.000
16.007	-	-	100	0.030	0.030	0.030
16.008	-	-	100	0.000	0.000	0.000
16.009	-	-	100	0.127	0.127	0.127
16.010	-	-	100	0.055	0.055	0.055
16.011	-	-	100	0.078	0.078	0.078
16.012	-	-	100	0.091	0.091	0.091
16.013	-	-	100	0.045	0.045	0.045
16.014	-	-	100	0.045	0.045	0.045
16.015	-	-	100	0.000	0.000	0.000
16.016	-	-	100	0.051	0.051	0.051
21.000	-	-	100	0.016	0.016	0.016
21.001	-	-	100	0.058	0.058	0.058
21.002	-	-	100	0.024	0.024	0.024
21.003	-	-	100	0.082	0.082	0.082
21.004	-	-	100	0.032	0.032	0.032
21.005	-	-	100	0.038	0.038	0.038
21.006	-	-	100	0.041	0.041	0.041
21.007	-	-	100	0.034	0.034	0.034
21.008	-	-	100	0.024	0.024	0.024
22.000	-	-	100	0.019	0.019	0.019
22.001	-	-	100	0.000	0.000	0.000
21.009	-	-	100	0.061	0.061	0.061
23.000	-	-	100	0.022	0.022	0.022
23.001	-	-	100	0.044	0.044	0.044
23.002	-	-	100	0.000	0.000	0.000
21.010	-	-	100	0.017	0.017	0.017
21.011	-	-	100	0.054	0.054	0.054
21.012	-	-	100	0.061	0.061	0.061
21.013	-	-	100	0.115	0.115	0.115
21.014	-	-	100	0.030	0.030	0.030
16.017	-	-	100	0.068	0.068	0.068
16.018	-	-	100	0.083	0.083	0.083
16.019	-	-	100	0.051	0.051	0.051
16.020	-	-	100	0.056	0.056	0.056
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Area Summary for Storm

Pipe Number	PIMP Type	PIMP Name	PIMP (%)	Gross Area (ha)	Imp. Area (ha)	Pipe Total (ha)
24.000	-	-	100	0.035	0.035	0.035
24.001	-	-	100	0.043	0.043	0.043
24.002	-	-	100	0.000	0.000	0.000
24.003	-	-	100	0.044	0.044	0.044
24.004	-	-	100	0.042	0.042	0.042
25.000	-	-	100	0.020	0.020	0.020
25.001	-	-	100	0.021	0.021	0.021
25.002	-	-	100	0.042	0.042	0.042
24.005	-	-	100	0.019	0.019	0.019
24.006	-	-	100	0.006	0.006	0.006
16.021	-	-	100	0.089	0.089	0.089
16.022	-	-	100	0.032	0.032	0.032
16.023	-	-	100	0.016	0.016	0.016
16.024	-	-	100	0.005	0.005	0.005
16.025	-	-	100	0.043	0.043	0.043
16.026	-	-	100	0.014	0.014	0.014
16.027	-	-	100	0.000	0.000	0.000
16.028	-	-	100	0.000	0.000	0.000
				Total	Total	Total
				2.694	2.694	2.694

Surcharged Outfall Details for Storm

Outfall Pipe Number	Outfall Name	C. Level (m)	I. Level (m)	Min I. Level (m)	D,L (mm)	W (mm)
---------------------	--------------	--------------	--------------	------------------	----------	--------

S16.028 S 17.500 16.999 17.000 0 0

Datum (m) 0.000 Offset (mins) 0

Time (mins)	Depth (m)	Time (mins)	Depth (m)	Time (mins)	Depth (m)	Time (mins)	Depth (m)	Time (mins)	Depth (m)
1440	17.500	5760	17.500	10080	17.500	14400	17.500	18720	17.500
2880	17.500	7200	17.500	11520	17.500	15840	17.500	20160	17.500
4320	17.500	8640	17.500	12960	17.500	17280	17.500		

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Simulation Criteria for Storm

Volumetric Runoff Coeff	0.750	Additional Flow - % of Total Flow	0.000
Areal Reduction Factor	1.000	MADD Factor * 10m ³ /ha Storage	2.000
Hot Start (mins)	0	Inlet Coefficient	0.800
Hot Start Level (mm)	0	Flow per Person per Day (l/per/day)	0.000
Manhole Headloss Coeff (Global)	0.500	Run Time (mins)	60
Foul Sewage per hectare (l/s)	0.000	Output Interval (mins)	1

Number of Input Hydrographs	0	Number of Storage Structures	5
Number of Online Controls	5	Number of Time/Area Diagrams	0
Number of Offline Controls	0	Number of Real Time Controls	0

Synthetic Rainfall Details

Rainfall Model	FSR	Profile Type	Summer
Return Period (years)	100	Cv (Summer)	0.750
Region	England and Wales	Cv (Winter)	0.840
M5-60 (mm)	19.900	Storm Duration (mins)	30
Ratio R	0.350		

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Online Controls for Storm

Hydro-Brake® Optimum Manhole: SHBRAKE PP5, DS/PN: S21.001, Volume (m³): 1.6

Unit Reference MD-SHE-0087-3500-1104-3500
Design Head (m) 1.104
Design Flow (l/s) 3.5
Flush-Flo™ Calculated
Objective Minimise upstream storage
Application Surface
Sump Available Yes
Diameter (mm) 87
Invert Level (m) 23.150
Minimum Outlet Pipe Diameter (mm) 100
Suggested Manhole Diameter (mm) 1200

Control Points	Head (m)	Flow (l/s)
Design Point (Calculated)	1.104	3.5
Flush-Flo™	0.334	3.5
Kick-Flo®	0.689	2.8
Mean Flow over Head Range	-	3.1

The hydrological calculations have been based on the Head/Discharge relationship for the Hydro-Brake® Optimum as specified. Should another type of control device other than a Hydro-Brake Optimum® be utilised then these storage routing calculations will be invalidated

Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)
0.100	2.7	1.200	3.6	3.000	5.6	7.000	8.3
0.200	3.3	1.400	3.9	3.500	6.0	7.500	8.6
0.300	3.5	1.600	4.2	4.000	6.4	8.000	8.9
0.400	3.5	1.800	4.4	4.500	6.7	8.500	9.1
0.500	3.4	2.000	4.6	5.000	7.1	9.000	9.4
0.600	3.2	2.200	4.8	5.500	7.4	9.500	9.6
0.800	3.0	2.400	5.0	6.000	7.7		
1.000	3.3	2.600	5.2	6.500	8.0		

Hydro-Brake® Optimum Manhole: SHBRAKE PP6, DS/PN: S22.001, Volume (m³): 1.6

Unit Reference MD-SHE-0055-1500-1176-1500
Design Head (m) 1.176
Design Flow (l/s) 1.5
Flush-Flo™ Calculated
Objective Minimise upstream storage
Application Surface
Sump Available Yes
Diameter (mm) 55
Invert Level (m) 22.923
Minimum Outlet Pipe Diameter (mm) 75
Suggested Manhole Diameter (mm) 1200

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<u>Hydro-Brake® Optimum Manhole: SHBRAKE PP6, DS/PN: S22.001, Volume (m³): 1.6</u>																																																																										
<table><tr><td>Control Points</td><td>Head (m)</td><td>Flow (l/s)</td></tr><tr><td>Design Point (Calculated)</td><td>1.176</td><td>1.5</td></tr><tr><td>Flush-Flo™</td><td>0.245</td><td>1.2</td></tr><tr><td>Kick-Flo®</td><td>0.496</td><td>1.0</td></tr><tr><td>Mean Flow over Head Range</td><td>-</td><td>1.2</td></tr></table>			Control Points	Head (m)	Flow (l/s)	Design Point (Calculated)	1.176	1.5	Flush-Flo™	0.245	1.2	Kick-Flo®	0.496	1.0	Mean Flow over Head Range	-	1.2																																																									
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<u>Hydro-Brake® Optimum Manhole: SHBRAKE PP7, DS/PN: S24.001, Volume (m³): 1.6</u>																																																																										
<table><tr><td>Unit Reference</td><td>MD-SHE-0061-1500-0797-1500</td></tr><tr><td>Design Head (m)</td><td>0.797</td></tr><tr><td>Design Flow (l/s)</td><td>1.5</td></tr><tr><td>Flush-Flo™</td><td>Calculated</td></tr><tr><td>Objective</td><td>Minimise upstream storage</td></tr><tr><td>Application</td><td>Surface</td></tr><tr><td>Sump Available</td><td>Yes</td></tr><tr><td>Diameter (mm)</td><td>61</td></tr><tr><td>Invert Level (m)</td><td>21.528</td></tr><tr><td>Minimum Outlet Pipe Diameter (mm)</td><td>75</td></tr><tr><td>Suggested Manhole Diameter (mm)</td><td>1200</td></tr></table>			Unit Reference	MD-SHE-0061-1500-0797-1500	Design Head (m)	0.797	Design Flow (l/s)	1.5	Flush-Flo™	Calculated	Objective	Minimise upstream storage	Application	Surface	Sump Available	Yes	Diameter (mm)	61	Invert Level (m)	21.528	Minimum Outlet Pipe Diameter (mm)	75	Suggested Manhole Diameter (mm)	1200																																																		
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Hydro-Brake® Optimum Manhole: SHBRAKE PP7, DS/PN: S24.001, Volume (m³): 1.6

Hydro-Brake® Optimum Manhole: SHBRAKE PP8, DS/PN: S25.001, Volume (m³): 1.6

Control Points	Head (m)	Flow (l/s)
Design Point (Calculated)	0.679	1.0
Flush-Flo™	0.213	1.0
Kick-Flo®	0.435	0.8
Mean Flow over Head Range	-	0.9

Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)
0.100	0.9	1.200	1.3	3.000	2.0	7.000	2.9
0.200	1.0	1.400	1.4	3.500	2.1	7.500	3.0
0.300	1.0	1.600	1.5	4.000	2.2	8.000	3.1
0.400	0.9	1.800	1.5	4.500	2.4	8.500	3.2
0.500	0.9	2.000	1.6	5.000	2.5	9.000	3.3
0.600	0.9	2.200	1.7	5.500	2.6	9.500	3.4
0.800	1.1	2.400	1.8	6.000	2.7		
1.000	1.2	2.600	1.8	6.500	2.8		

Unit Reference	MD-SHE-0265-3960-1150-3960
Design Head (m)	1.150
Design Flow (l/s)	39.6

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Hydro-Brake® Optimum Manhole: SHBRAKE BASIN L, DS/PN: S16.028, Volume (m³): 33.6 Flush-Flo™ Objective Minimise upstream storage Application Sump Available Diameter (mm) Invert Level (m) Minimum Outlet Pipe Diameter (mm) Suggested Manhole Diameter (mm) Calculated Surface Yes 265 18.949 300 1800 Control Points Head (m) Flow (l/s) Design Point (Calculated) Flush-Flo™ Kick-Flo® Mean Flow over Head Range 1.150 0.428 0.852 - 39.6 39.6 34.3 32.8 The hydrological calculations have been based on the Head/Discharge relationship for the Hydro-Brake® Optimum as specified. Should another type of control device other than a Hydro-Brake Optimum® be utilised then these storage routing calculations will be invalidated Depth (m) Flow (l/s) Depth (m) Flow (l/s) Depth (m) Flow (l/s) Depth (m) Flow (l/s) 0.100 8.4 1.200 40.4 3.000 62.9 7.000 95.0 0.200 27.2 1.400 43.5 3.500 67.8 7.500 98.2 0.300 38.7 1.600 46.4 4.000 72.3 8.000 101.4 0.400 39.6 1.800 49.1 4.500 76.6 8.500 104.4 0.500 39.4 2.000 51.7 5.000 80.6 9.000 107.4 0.600 38.8 2.200 54.1 5.500 84.4 9.500 110.2 0.800 36.0 2.400 56.5 6.000 88.1 1.000 37.0 2.600 58.7 6.500 91.6 ©1982-2020 Innovyze		

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Date 25/05/2021 12:29 File 2021-05-24 Basin L.MDX	Designed by BASPC1 Checked by																																																																																																									
Innovyze	Network 2020.1																																																																																																									
<u>Storage Structures for Storm</u> <u>Porous Car Park Manhole: SPP5, DS/PN: S21.000</u> <table><tr><td>Infiltration Coefficient Base (m/hr)</td><td>0.00000</td><td>Width (m)</td><td>10.0</td></tr><tr><td>Membrane Percolation (mm/hr)</td><td>1000</td><td>Length (m)</td><td>16.2</td></tr><tr><td>Max Percolation (l/s)</td><td>45.0</td><td>Slope (1:X)</td><td>100.0</td></tr><tr><td>Safety Factor</td><td>2.0</td><td>Depression Storage (mm)</td><td>5</td></tr><tr><td>Porosity</td><td>0.30</td><td>Evaporation (mm/day)</td><td>3</td></tr><tr><td>Invert Level (m)</td><td>23.754</td><td>Membrane Depth (mm)</td><td>0</td></tr></table> <u>Porous Car Park Manhole: SPP6, DS/PN: S22.000</u> <table><tr><td>Infiltration Coefficient Base (m/hr)</td><td>0.00000</td><td>Width (m)</td><td>10.0</td></tr><tr><td>Membrane Percolation (mm/hr)</td><td>1000</td><td>Length (m)</td><td>18.7</td></tr><tr><td>Max Percolation (l/s)</td><td>51.9</td><td>Slope (1:X)</td><td>60.0</td></tr><tr><td>Safety Factor</td><td>2.0</td><td>Depression Storage (mm)</td><td>5</td></tr><tr><td>Porosity</td><td>0.30</td><td>Evaporation (mm/day)</td><td>3</td></tr><tr><td>Invert Level (m)</td><td>23.599</td><td>Membrane Depth (mm)</td><td>0</td></tr></table> <u>Porous Car Park Manhole: SPP7, DS/PN: S24.000</u> <table><tr><td>Infiltration Coefficient Base (m/hr)</td><td>0.00000</td><td>Width (m)</td><td>11.0</td></tr><tr><td>Membrane Percolation (mm/hr)</td><td>1000</td><td>Length (m)</td><td>31.9</td></tr><tr><td>Max Percolation (l/s)</td><td>97.5</td><td>Slope (1:X)</td><td>100.0</td></tr><tr><td>Safety Factor</td><td>2.0</td><td>Depression Storage (mm)</td><td>5</td></tr><tr><td>Porosity</td><td>0.30</td><td>Evaporation (mm/day)</td><td>3</td></tr><tr><td>Invert Level (m)</td><td>21.825</td><td>Membrane Depth (mm)</td><td>0</td></tr></table> <u>Porous Car Park Manhole: SPP8, DS/PN: S25.000</u> <table><tr><td>Infiltration Coefficient Base (m/hr)</td><td>0.00000</td><td>Width (m)</td><td>11.0</td></tr><tr><td>Membrane Percolation (mm/hr)</td><td>1000</td><td>Length (m)</td><td>17.7</td></tr><tr><td>Max Percolation (l/s)</td><td>54.1</td><td>Slope (1:X)</td><td>100.0</td></tr><tr><td>Safety Factor</td><td>2.0</td><td>Depression Storage (mm)</td><td>5</td></tr><tr><td>Porosity</td><td>0.30</td><td>Evaporation (mm/day)</td><td>3</td></tr><tr><td>Invert Level (m)</td><td>20.736</td><td>Membrane Depth (mm)</td><td>0</td></tr></table> <u>Tank or Pond Manhole: SBASIN L, DS/PN: S16.027</u> Invert Level (m) 19.200 <table><tr><td>Depth (m)</td><td>Area (m²)</td><td>Depth (m)</td><td>Area (m²)</td></tr><tr><td>0.000</td><td>1788.0</td><td>0.900</td><td>3216.0</td></tr></table>			Infiltration Coefficient Base (m/hr)	0.00000	Width (m)	10.0	Membrane Percolation (mm/hr)	1000	Length (m)	16.2	Max Percolation (l/s)	45.0	Slope (1:X)	100.0	Safety Factor	2.0	Depression Storage (mm)	5	Porosity	0.30	Evaporation (mm/day)	3	Invert Level (m)	23.754	Membrane Depth (mm)	0	Infiltration Coefficient Base (m/hr)	0.00000	Width (m)	10.0	Membrane Percolation (mm/hr)	1000	Length (m)	18.7	Max Percolation (l/s)	51.9	Slope (1:X)	60.0	Safety Factor	2.0	Depression Storage (mm)	5	Porosity	0.30	Evaporation (mm/day)	3	Invert Level (m)	23.599	Membrane Depth (mm)	0	Infiltration Coefficient Base (m/hr)	0.00000	Width (m)	11.0	Membrane Percolation (mm/hr)	1000	Length (m)	31.9	Max Percolation (l/s)	97.5	Slope (1:X)	100.0	Safety Factor	2.0	Depression Storage (mm)	5	Porosity	0.30	Evaporation (mm/day)	3	Invert Level (m)	21.825	Membrane Depth (mm)	0	Infiltration Coefficient Base (m/hr)	0.00000	Width (m)	11.0	Membrane Percolation (mm/hr)	1000	Length (m)	17.7	Max Percolation (l/s)	54.1	Slope (1:X)	100.0	Safety Factor	2.0	Depression Storage (mm)	5	Porosity	0.30	Evaporation (mm/day)	3	Invert Level (m)	20.736	Membrane Depth (mm)	0	Depth (m)	Area (m²)	Depth (m)	Area (m²)	0.000	1788.0	0.900	3216.0
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Micro
Drainage

Designed by BASPC1
Checked by

Network 2020.1

PN	US/MH Name	Storm	Return Period	Climate Change	First (X) Surcharge	First (Y) Flood	First (Z) Overflow
S16.008	S101	15 Winter	100	+40%	100/15 Summer		
S16.009	S102	30 Winter	100	+40%	100/15 Summer		
S16.010	S103	30 Winter	100	+40%	100/15 Summer		
S16.011	S104	30 Winter	100	+40%	100/15 Summer		
S16.012	S105	30 Winter	100	+40%	100/15 Summer	100/15 Winter	
S16.013	S106	30 Winter	100	+40%	100/15 Summer		
S16.014	S107	30 Winter	100	+40%	100/15 Summer		
S16.015	S108	30 Winter	100	+40%	100/15 Summer		
S16.016	S109	30 Winter	100	+40%	100/15 Summer		
S21.000	SPP5	60 Winter	100	+40%	100/15 Summer		
S21.001	SHBRAKE PP5	15 Winter	100	+40%	100/15 Summer		
S21.002	S112	15 Winter	100	+40%	100/15 Summer		
S21.003	S113	15 Winter	100	+40%	100/15 Summer		
S21.004	S114	15 Winter	100	+40%	100/15 Summer		
S21.005	S115	30 Winter	100	+40%	100/15 Summer	100/30 Winter	
S21.006	S116	30 Winter	100	+40%	100/15 Summer		
S21.007	S117	30 Winter	100	+40%	100/15 Summer		
S21.008	S118	30 Winter	100	+40%	100/15 Summer		
S22.000	SPP6	60 Winter	100	+40%	100/15 Winter		
S22.001	SHBRAKE PP6	60 Winter	100	+40%	100/15 Summer		
S21.009	S121	30 Winter	100	+40%	100/15 Summer		
S23.000	S122	15 Winter	100	+40%	100/15 Summer		
S23.001	S123	15 Winter	100	+40%	100/15 Summer		
S23.002	S124	15 Winter	100	+40%			
S21.010	S125	30 Winter	100	+40%	100/15 Summer		
S21.011	S126	30 Winter	100	+40%	100/15 Summer		
S21.012	S127	30 Winter	100	+40%	100/15 Summer		
S21.013	S128	30 Winter	100	+40%	100/15 Summer		
S21.014	S129	30 Winter	100	+40%	100/15 Summer		
S16.017	S130	30 Winter	100	+40%	100/15 Summer		
S16.018	S131	30 Winter	100	+40%	100/15 Summer		
S16.019	S132	30 Winter	100	+40%	100/15 Summer		
S16.020	S133	30 Winter	100	+40%	100/15 Summer		
S24.000	SPP7	180 Winter	100	+40%	100/15 Summer		
S24.001	SHBRAKE PP7	180 Winter	100	+40%	100/15 Summer		
S24.002	S136	30 Winter	100	+40%	100/30 Winter		
S24.003	S137	30 Winter	100	+40%	100/15 Summer		
S24.004	S138	30 Winter	100	+40%	100/15 Summer		
S25.000	SPP8	180 Winter	100	+40%	100/15 Summer		
S25.001	SHBRAKE PP8	180 Winter	100	+40%	100/15 Summer		
S25.002	S141	30 Winter	100	+40%	100/15 Summer	100/30 Winter	
S24.005	S142	30 Winter	100	+40%	100/15 Summer	100/30 Winter	
S24.006	S143	30 Winter	100	+40%	100/15 Summer		
S16.021	S144	30 Winter	100	+40%	100/15 Summer		
S16.022	S145	30 Winter	100	+40%	100/15 Summer		
S16.023	S146	30 Winter	100	+40%	100/15 Summer		
S16.024	S147	30 Winter	100	+40%	100/15 Summer		
S16.025	S148	30 Winter	100	+40%	100/15 Summer		
S16.026	S149	30 Winter	100	+40%	100/15 Summer		

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Summary of Critical Results by Maximum Level (Rank 1) for Storm

PN	US/MH Name	Overflow Act.	Water Level (m)	Surcharged Depth (m)	Flooded Volume (m³)	Flow / Cap.	Overflow (l/s)	Half Drain Time (mins)	Pipe Flow (l/s)
S16.026	S149		20.365	0.334	0.000	1.78			623.7

PN	US/MH		Level
	Name	Status	Exceeded
S16.008	S101	SURCHARGED	
S16.009	S102	FLOOD RISK	
S16.010	S103	FLOOD RISK	
S16.011	S104	FLOOD RISK	
S16.012	S105	FLOOD	2
S16.013	S106	FLOOD RISK	
S16.014	S107	SURCHARGED	
S16.015	S108	SURCHARGED	
S16.016	S109	SURCHARGED	
S21.000	SPP5	SURCHARGED	
S21.001	SHBRAKE PP5	FLOOD RISK	
S21.002	S112	SURCHARGED	
S21.003	S113	FLOOD RISK	
S21.004	S114	FLOOD RISK	
S21.005	S115	FLOOD	1
S21.006	S116	FLOOD RISK	
S21.007	S117	FLOOD RISK	
S21.008	S118	SURCHARGED	
S22.000	SPP6	SURCHARGED	
S22.001	SHBRAKE PP6	SURCHARGED	
S21.009	S121	SURCHARGED	
S23.000	S122	SURCHARGED	
S23.001	S123	SURCHARGED	
S23.002	S124	OK	
S21.010	S125	SURCHARGED	
S21.011	S126	SURCHARGED	
S21.012	S127	SURCHARGED	
S21.013	S128	FLOOD RISK	
S21.014	S129	SURCHARGED	
S16.017	S130	SURCHARGED	
S16.018	S131	SURCHARGED	
S16.019	S132	SURCHARGED	
S16.020	S133	FLOOD RISK	
S24.000	SPP7	SURCHARGED	
S24.001	SHBRAKE PP7	SURCHARGED	
S24.002	S136	SURCHARGED	
S24.003	S137	SURCHARGED	
S24.004	S138	FLOOD RISK	
S25.000	SPP8	SURCHARGED	
S25.001	SHBRAKE PP8	SURCHARGED	
S25.002	S141	FLOOD	1
S24.005	S142	FLOOD	1

APPENDIX C

Water Management Strategy