



Title: 501 Bursledon Road

Date: 21st August 2026

Authored: K. Gyba

Revision: P01

1.0 Introduction

1.1.1 SWJ Consulting Engineers have been appointed by Chessel Support Services to produce a drainage strategy for a new residential development at 501 Bursledon Road, Southampton.

1.1.2 This technical note is prepared in accordance with the requirements of the National Planning Policy Framework (NPPF) published by the Department of Communities and Local Government. Section 14 of the NPPF and the associated Planning Practice Guidance sets out the framework for planning decisions made by the local, regional, and national government and the Environment Agency (EA).

2.0 Drainage Design

2.1 Surface Water Drainage Hierarchy

2.1.1 New surface water drainage will be required to drain surface water runoff from the new dwelling.

2.1.2 It is a requirement of the National SuDS Standards that any development should not result in an increase in surface water runoff, and where possible, should demonstrate betterment in terms of rate and volumes of surface water runoff. Generally, the aim should be to discharge surface water run-off in accordance with the following hierarchy of surface water disposal options wherever reasonably practicable, (in order of preference):

Discharge Method	Feasibility	Notes
Rainwater re-use (rainwater harvesting/greywater recycling)	✓	Rainwater butts can be used for irrigation purposes.
An adequate soakaway or other infiltration system	X	It is not possible to meet the minimum 5m offset requirement from buildings and adopted highways.
To a surface water body (e.g. an ordinary watercourse)	X	There are no surface water bodies near the proposed development site.
To a surface water sewer, highway drain, or other drainage system	✓	Southern Water asset map shows a Surface Water sewer located directly outside of the proposed development site. It is therefore a suitable method of discharge from this development.
To a combined sewer	X	Not required.

Table 1: Drainage Hierarchy

2.2 Surface Water Drainage Strategy

2.2.1 The proposed catchments on site consist of a proposed building (250m²) and the associated car-parking (200m²). The total impermeable area onsite measures 450m².

- 2.2.2 In line with national policies, the site should aim to limit runoff rates to as close as possible to Greenfield Runoff rates. The greenfield runoff rate for the site (Q_{bar}) has been measured as 0.1 l/s – see calculations in Appendices. In order to avoid blockages, a minimum discharge rate of 2.0 l/s will be used as a discharge rate from site. Restriction will be achieved via use of a Hydrobrake positioned at the outfall manhole.
- 2.2.3 The site will aim to utilise rainwater butts for rainwater harvesting, to aid with irrigation requirements and reduce potable water usage. Surface water from the roof will be directed towards the below ground system, which will route the water towards the permeable paving sub-base. Approximately 200m² of permeable paving will be provided. Restriction of runoff from site, for all storms, up to and including 1in100+45% climate change events, will require a min. depth of 300mm of Type 3 sub-base (30% void) to provide the necessary storage to restrict flows to 2.0 l/s. In line with BS 7533-13, the permeable paving sub-base will be 350mm deep, to accommodate car-park loading scenario, hence there will be some redundancy in the system.
- 2.2.4 A copy of the drainage calculations is included within the appendices of this technical note.
- 2.2.5 A copy of the drainage strategy layout is also included within the appendices of this technical note. Extract shown below. The drainage layout shows routes of overland flows, to ensure water is managed safely during an extreme rainfall scenario / localised drainage system failure. As shown, flows will be routed around the proposed building, following the natural topography of the site in the southern direction. Flows within the neighbouring highway will continue to flow along the kerb line.

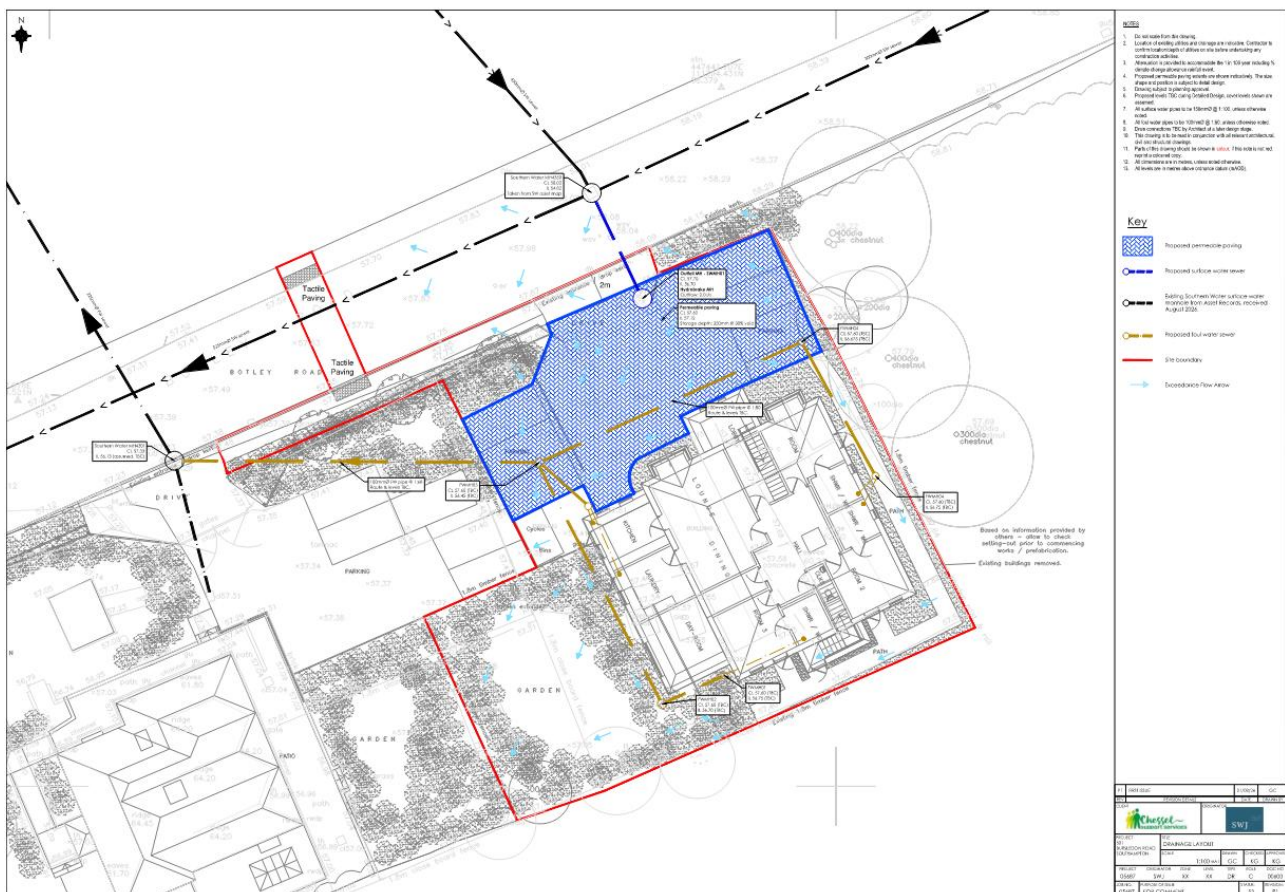


Figure 1: Proposed Drainage Strategy

2.3 Water Quality

- 2.3.1 The surface water management should incorporate sustainable drainage techniques to restrict surface water discharge from the site and improve water quality of the run-off. There are a wide range of techniques that can be applied including source control, online systems, and outlet controls. CIRIA have published a SuDS manual, which details a number of systems along with guidance on their application and design.
- 2.3.2 SuDS systems typically rely on either infiltration or attenuation to reduce peak flows and volume discharge and filtration systems to remove pollutants or solids from the effluent.
- 2.3.3 The existing ground conditions appear to suggest infiltration methods will be a viable solution for the discharge of surface water flows from the proposed development.
- 2.3.4 As a means of analysing whether the SuDS measures utilised for the site will provide adequate water treatment to the surface water runoff the CIRIA SuDS manual specifies a Simple Index Approach. This approach gives different land uses certain pollution hazard indices (Table 26.2 CIRIA Manual, while SuDS features are given mitigation indices (Table 26.4 CIRIA Manual). If the SuDS indices are greater or equal that the pollution hazard indices the water treatment within the system is deemed adequate.
- 2.3.5 The impermeable hardstanding areas of the site can be split into four distinct areas:
- Building Roof Areas
 - Carparking Areas
- 2.3.6 The figure below shows the relevant pollution hazard indices for different land use classifications (Table 26.2 CIRIA SuDS Manual).

TABLE 26.2 Pollution hazard indices for different land use classifications				
Land use	Pollution hazard level	Total suspended solids (TSS)	Metals	Hydro-carbons
Residential roofs	Very low	0.2	0.2	0.05
Other roofs (typically commercial/ industrial roofs)	Low	0.3	0.2 (up to 0.8 where there is potential for metals to leach from the roof)	0.05
Individual property driveways, residential car parks, low traffic roads (eg cul de sacs, homezones and general access roads) and non-residential car parking with infrequent change (eg schools, offices) ie < 300 traffic movements/day	Low	0.5	0.4	0.4
Commercial yard and delivery areas, non-residential car parking with frequent change (eg hospitals, retail), all roads except low traffic roads and trunk roads/motorways ¹	Medium	0.7	0.6	0.7
Sites with heavy pollution (eg haulage yards, lorry parks, highly frequented lorry approaches to industrial estates, waste sites), sites where chemicals and fuels (other than domestic fuel oil) are to be delivered, handled, stored, used or manufactured; industrial sites; trunk roads and motorways ¹	High	0.8 ²	0.8 ²	0.9 ²

Table 26.2 CIRIA SuDS Manual

2.3.7 The parking areas will provide the highest hazard indices. These are considered a low Pollution Hazard Level, with levels of TSS – 0.5, Metals 0.4, and Hydrocarbons – 0.4.

2.3.8 All surface water will pass through at least one SuDS features prior to being discharged with all water passing through permeable paving. The figure below shows the indicative SuDS mitigation indices for discharges to surface waters (Table 26.4 CIRIA SuDS Manual).

TABLE 26.4 Indicative SuDS mitigation indices for discharges to groundwater			
Characteristics of the material overlying the proposed infiltration surface, through which the runoff percolates¹	TSS	Metals	Hydrocarbons
A layer of dense vegetation underlain by a soil with good contaminant attenuation potential ² of at least 300 mm in depth ³	0.6 ⁴	0.5	0.6
A soil with good contaminant attenuation potential ² of at least 300 mm in depth ³	0.4 ⁴	0.3	0.3
Infiltration trench (where a suitable depth of filtration material is included that provides treatment, ie graded gravel with sufficient smaller particles but not single size coarse aggregate such as 20 mm gravel) underlain by a soil with good contaminant attenuation potential ² of at least 300 mm in depth ³	0.4 ⁴	0.4	0.4
Constructed permeable pavement (where a suitable filtration layer is included that provides treatment, and including a geotextile at the base separating the foundation from the subgrade) underlain by a soil with good contaminant attenuation potential ² of at least 300 mm in depth ³	0.7	0.6	0.7
Bioretention underlain by a soil with good contaminant attenuation potential ² of at least 300 mm in depth ³	0.8 ⁴	0.8	0.8
Proprietary treatment systems ^{5, 6}	These must demonstrate that they can address each of the contaminant types to acceptable levels for inflow concentrations relevant to the contributing drainage area.		

Table 26.3 CIRIA SuDS Manual

2.3.9 As can be seen from the tables above, the permeable paving will provide sufficient water cleansing to any surface water produced by the site. Summary is provided in table below.

	TSS	Metals	Hydrocarbons
Pollution	0.5	0.4	0.4
Treatment (perm paving)	0.7	0.6	0.7
Summary: Treatment indices exceed pollution indices; hence sufficient treatment is provided.			

2.3.10 During the construction phase of the development, the contractor shall ensure that appropriate provisions are put in place to ensure that the water quality of the surface water discharge from the site is maintained. This will involve measures to prevent silts or other construction waste being conveyed offsite through surface water runoff.

2.4 Foul Water Drainage

- 2.4.1 A new foul water network will be required to service the proposed development. The new network will collect and convey foul water discharge from the development to an existing Southern Water manhole, as shown on the drainage layout.
- 2.4.2 A pre-development application with Southern Water will be submitted to confirm capacity. New connection will be subject to a S106 application, prior to construction.
- 2.4.3 A copy of the drainage strategy is included within the appendices of this technical note.

2.5 Management and Maintenance

- 2.5.1 All drainage infrastructure to be constructed within the proposed development site will be privately owned and not adopted by the local water authority.
- 2.5.2 All drainage features will be managed and maintained privately by owner.
- 2.5.3 As the scheme is progressed, management and maintenance practices for taking care of the SuDS/Drainage infrastructure will be constantly reviewed and updated with a final confirmed plan to be detailed at the completion of construction and included within the O&M Manuals for the site.
- 2.5.4 SuDS features will be managed in accordance with the guidelines in CIRIA C753, Chapter 32. These are detailed below:

TABLE 32.1 Typical key SuDS components operation and maintenance activities (for full specifications, see Chapters 11–23)

Operation and maintenance activity	SuDS component												
	Pond	Wetland	Detention basin	Infiltration basin	Soakaway	Infiltration trench	Filter drain	Modular storage	Pervious pavement	Swale/bioretention/ trees	Filter strip	Green roofs	Proprietary treatment systems
Regular maintenance													
Inspection	■	■	■	■	■	■	■	■	■	■	■	■	■
Litter and debris removal	■	■	■	■	□	■	■	□	■	■	■		□
Grass cutting	■	■	■	■	□	■	■	□	□	■	■		
Weed and invasive plant control	□	□	□	□		□	□		□		□	■	
Shrub management (including pruning)	□	□	□	□					□	□	□		
Shoreline vegetation management	■	■	□										
Aquatic vegetation management	■	■	□										
Occasional maintenance													
Sediment management¹	■	■	■	■	■	■	■	■	■	■	■		■
Vegetation replacement	□	□	□	□						□	□	■	
Vacuum sweeping and brushing									■				
Remedial maintenance													
Structure rehabilitation /repair	□	□	□	□	□	□	□	□	□	□	□	□	
Infiltration surface reconditioning				□	□	□	□		□	□	□		

Key

- will be required
□ may be required

Notes

- ¹ Sediment should be collected and managed in pre-treatment systems, upstream of the main device.

Table 32.1 CIRIA SuDS Manual Maintenance Activities

General Requirements

Regular Maintenance	Frequency
Litter Collect all litter or other debris and remove from site at each site visit.	Monthly or when deemed necessary
Grass maintenance Amenity Grass – Mow all grass verges, paths and amenity grass at 35- 50mm with 75mm max. All cuttings to remain in situ.	Monthly or when deemed necessary

Hydrobrake

Regular Maintenance	Frequency
Inspection Chamber Remove cover to inspect and note any high-water levels, re-inspect 24hrs later to evaluate reduction of water levels. Inspect ensuring that water is flowing appropriately through the flow control device and there are no obstructions to the flow of water immediately upstream or within the device. Remove debris and silt where necessary.	Monthly or following severe storms, within first year. Monthly for 3 months, then every 6 months.
Reactive Maintenance	Frequency
If a blockage within the hydrobrake unit occurs, it will need to be either jetted or replaced depending on the severity of the blockage.	As required

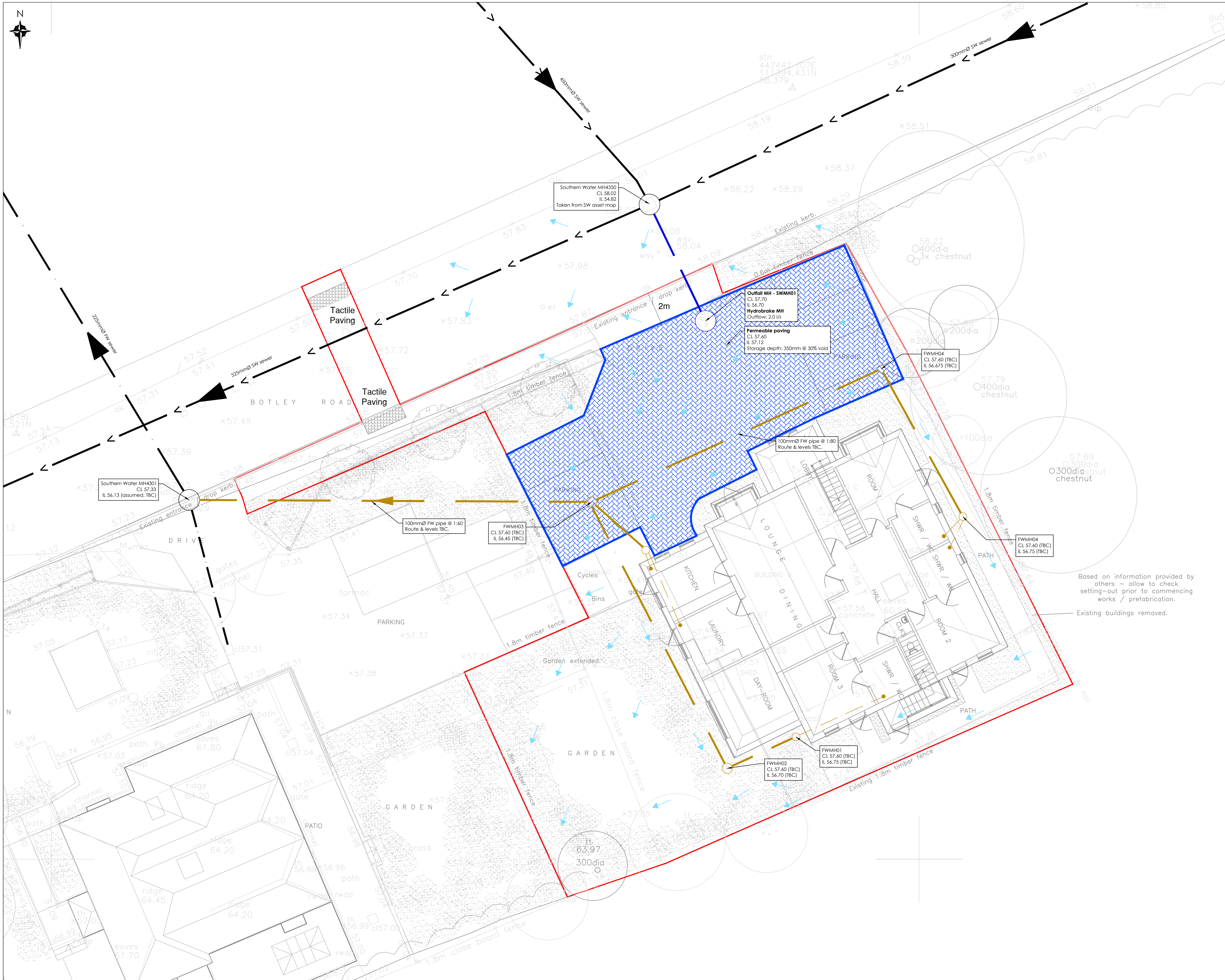
Permeable Paving

Regular Maintenance	Frequency
Surface Protection Remove litter and debris. Brush or suction sweep surface to remove silt build up and replace grit as required	Monthly Annually
Bedding Replacement Lift blocks and remove bedding material. Clean geotextile and replace bedding material with new silt-free granular material.	Every 10 years or as required
Reactive Maintenance	Frequency
Brush or suction sweep to remove any materials left on surface. Lift and re-bed blocks where movement has occurred. In case of settlement, full reconstruction and compaction of sub-base may be required. Sub-grade should be checked to washout of fines.	As required

3.0 Conclusion

- 3.1.1 It has been shown that the development as designed complies with the planning requirements, following the drainage discharge hierarchy, implementing SuDS and restricting the runoff rates from the development to as close as possible to greenfield runoff rates, for all storms, up to and including 1in100+45% climate change.
- 3.1.2 The proposed design does not increase flood risk elsewhere and provides a suitable discharge method for both foul and surface water produced by the site.
- 3.1.3 Consideration has been given to ongoing maintenance arrangements for proposed SuDS features, overland flows and water quality.

Appendices



- NOTES**
1. Do not scale from this drawing.
 2. Location of existing utilities and drainage are indicative. Contractor to confirm location/depth of utilities on site before undertaking any construction activities.
 3. Attenuation is provided to accommodate the 1 in 100 year including % climate change allowance rainfall event.
 4. Proposed permeable paving extents are shown indicatively. The size, shape and position is subject to detail design.
 5. Drawing subject to planning approval.
 6. Proposed levels TBC during Detailed Design, cover levels shown are assumed.
 7. All surface water pipes to be 150mmØ @ 1:100, unless otherwise noted.
 8. All foul water pipes to be 100mmØ @ 1:80, unless otherwise noted.
 9. Drain connections TBC by Architect at a later design stage.
 10. This drawing is to be read in conjunction with all relevant architectural, civil and structural drawings.
 11. Parts of this drawing should be shown in **colour**, if this note is not red, reprint a coloured copy.
 12. All dimensions are in metres, unless noted otherwise.
 13. All levels are in metres above ordnance datum (mAOD).

Key

- Proposed permeable paving
- Proposed surface water sewer
- Existing Southern Water surface water manhole from Asset Records, received August 2026.
- Proposed foul water sewer
- Site boundary
- Exceedance Flow Arrow

Based on information provided by others – allow to check setting-out prior to commencing works / prefabrication.

Existing buildings removed.

P1	FIRST ISSUE	21/08/26	GC
REV	REVISION DETAILS	DATE	DRAWN BY
CLIENT	 		
PROJECT	501 BURSLEDON ROAD SOUTHAMPTON	TITLE	DRAINAGE LAYOUT
SCALE	1:100 @A1	DRAWN	GC
ORIGINATOR	SWJ	CHECKED	KG
PROJECT	05687	ROLE	C
JOB NO.	05687	STATUS	S3
PURPOSE OF ISSUE	FOR COMMENT	REVISION	P1

Project:	Date: 20/08/2026			
	Designed by: Karol.Gyba	Checked by:	Approved By:	
Report Details:	Company Address:			
Type: Inflows Storm Phase: Phase				



Permeable paving

Type : Catchment Area

Area (ha)	0.02
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Dynamic Sizing

Runoff Method	Time of Concentration
Summer Volumetric Runoff	0.750
Winter Volumetric Runoff	0.840
Time of Concentration (mins)	5
Percentage Impervious (%)	100



Building

Type : Catchment Area

Area (ha)	0.025
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Dynamic Sizing

Runoff Method	Time of Concentration
Summer Volumetric Runoff	0.750
Winter Volumetric Runoff	0.840
Time of Concentration (mins)	5
Percentage Impervious (%)	100

Project:	Date: 20/08/2026		
	Designed by: Karol.Gyba	Checked by:	Approved By:
Report Details:	Company Address:		
Type: Stormwater Controls			
Storm Phase: Phase			



Porous Paving

Type : Porous Paving

Dimensions

Exceedance Level (m)	57.600
Depth (m)	0.480
Base Level (m)	57.120
Paving Layer Depth (mm)	130
Membrane Percolation (m/hr)	500.0
Porosity (%)	30
Length (m)	10.000
Long. Slope (1:X)	500.00
Width (m)	20.000
Total Volume (m³)	21.000

Inlets

Inlet

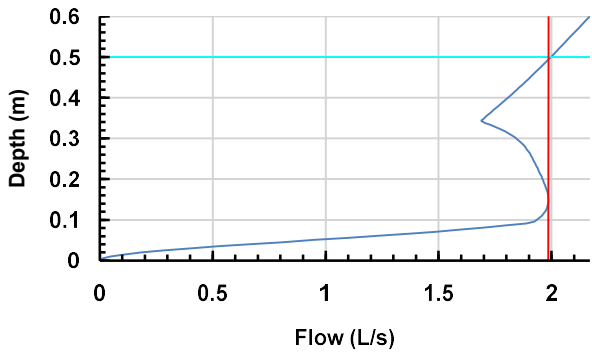
Inlet Type	Lateral Inflow
Incoming Item(s)	Building
	Permeable paving
Bypass Destination	(None)
Capacity Type	No Restriction

Outlets

Outlet

Outgoing Connection	(None)
Outlet Type	Hydro-Brake®
Invert Level (m)	57.120
Design Depth (m)	0.500
Design Flow (L/s)	2.0
Objective	Minimise Upstream Storage Requirements
Application	Surface Water Only
Sump Available	☒

Unit Reference	SHE-0074-2000-0500-2000
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Advanced

Conductivity (m/hr)	5000.0
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Project:	Date: 20/08/2026		
	Designed by: Karol.Gyba	Checked by:	Approved By:
	Company Address:		
Report Details: Type: Inflow Summary Storm Phase: Phase			



Inflow Label	Connected To	Flow (L/s)	Runoff Method	Area (ha)	Percentage Impervious (%)	Urban Creep (%)	Adjusted Percentage Impervious (%)	Area Analysed (ha)
Building	Porous Paving		Time of Concentration	0.025	100	0	100	0.025
Permeable paving	Porous Paving		Time of Concentration	0.020	100	0	100	0.020
TOTAL		0.0		0.045				0.045

Project:	Date: 20/08/2026			
	Designed by: Karol.Gyba	Checked by:	Approved By:	
Report Title: Rainfall Analysis Criteria	Company Address:			

Runoff Type	Dynamic
Output Interval (mins)	5
Time Step	Default
Urban Creep	Apply Global Value
Urban Creep Global Value (%)	0
Junction Flood Risk Margin (mm)	300
Perform No Discharge Analysis	☐

Rainfall	
FSR	Type: FSR
Region	England And Wales
M5-60 (mm)	19.6
Ratio R	0.350
Summer	☒
Winter	☒

Return Period

Return Period (years)	Increase Rainfall (%)
1.0	0.000
30.0	40.000
100.0	45.000

Storm Durations

Duration (mins)	Run Time (mins)
15	30
30	60
60	120
120	240
180	360
240	480
360	720
480	960
600	1200
720	1440
960	1920
1440	2880
2160	4320
2880	5760
4320	8640
5760	11520
7200	14400
8640	17280
10080	20160

Project:	Date: 20/08/2026			
	Designed by: Karol Gyba	Checked by:	Approved By:	
Report Title: UK and Ireland Rural Runoff Calculator	Company Address:			

ICP SUDS / IH 124

Details

Method	ICP SUDS
Area (ha)	0.045
SAAR (mm)	800.0
Soil	0.3
Region	Region 7
Urban	0
Return Period (years)	0

Results

Region	QBAR Rural (L/s)	QBAR Urban (L/s)	Q 1 (years) (L/s)	Q 30 (years) (L/s)	Q 100 (years) (L/s)
Region 7	0.1	0.1	0.1	0.2	0.3

Project:	Date: 20/08/2026		
	Designed by: Karol.Gyba	Checked by:	Approved By:
Report Details: Type: Inflows Summary Storm Phase: Phase	Company Address:		





FSR: 1 years: Increase Rainfall (%): +0: Critical Storm Per Item: Rank By: Max. Inflow

Inflow	Storm Event	Inflow Area (ha)	Max. Inflow (L/s)	Total Inflow Volume (m³)
Permeable paving	FSR: 1 years: +0 %: 15 mins: Winter	0.02	2.5	1.218
Building	FSR: 1 years: +0 %: 15 mins: Winter	0.03	3.2	1.524

Project:	Date: 20/08/2026			
	Designed by:	Checked by:	Approved By:	
	Karol.Gyba			
Report Details: Type: Inflows Summary Storm Phase: Phase		Company Address:		



FSR: 30 years: Increase Rainfall (%): +40: Critical Storm Per Item: Rank By: Max. Inflow

Inflow	Storm Event	Inflow Area (ha)	Max. Inflow (L/s)	Total Inflow Volume (m³)
Permeable paving	FSR: 30 years: +40 %: 15 mins: Winter	0.02	8.7	4.183
Building	FSR: 30 years: +40 %: 15 mins: Winter	0.03	10.9	5.231

Project:	Date: 20/08/2026		
	Designed by: Karol.Gyba	Checked by:	Approved By:
Report Details: Type: Inflows Summary Storm Phase: Phase	Company Address:		





FSR: 100 years: Increase Rainfall (%): +45: Critical Storm Per Item: Rank By: Max. Inflow

Inflow	Storm Event	Inflow Area (ha)	Max. Inflow (L/s)	Total Inflow Volume (m³)
Permeable paving	FSR: 100 years: +45 %: 15 mins: Winter	0.02	11.7	5.609
Building	FSR: 100 years: +45 %: 15 mins: Winter	0.03	14.6	7.013

Project:	Date: 20/08/2026		
	Designed by: Karol.Gyba	Checked by:	Approved By:
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase	Company Address:		





FSR: 1 years: Increase Rainfall (%): +0: Critical Storm Per Item: Rank By: Max. Avg. Depth

Stormwater Control	Storm Event	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Residant Volume (m³)	Max. Flooded Volume (m³)	Total Lost Volume (m³)	Max. Outflow (L/s)	Total Discharge Volume (m³)	Percentage Available (%)	Status
Porous Paving	FSR: 1 years: +0 %: 60 mins: Winter	57.188	57.176	0.048	0.056	3.0	3.122	0.000	0.000	1.1	3.679	85.131	OK

Project:	Date: 20/08/2026			
	Designed by: Karol.Gyba	Checked by:	Approved By:	
	Report Details: Type: Stormwater Controls Summary Storm Phase: Phase			
Company Address:				



FSR: 30 years: Increase Rainfall (%): +40: Critical Storm Per Item: Rank By: Max. Avg. Depth

Stormwater Control	Storm Event	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Residant Volume (m³)	Max. Flooded Volume (m³)	Total Lost Volume (m³)	Max. Outflow (L/s)	Total Discharge Volume (m³)	Percentage Available (%)	Status
Porous Paving	FSR: 30 years: +40 %: 120 mins: Winter	57.326	57.319	0.186	0.199	6.7	11.551	0.000	0.000	2.0	18.522	44.996	OK

Project:	Date: 20/08/2026		
	Designed by: Karol.Gyba	Checked by:	Approved By:
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase		Company Address:	





FSR: 100 years: Increase Rainfall (%): +45: Critical Storm Per Item: Rank By:
Max. Avg. Depth

Stormwater Control	Storm Event	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Residant Volume (m³)	Max. Flooded Volume (m³)	Total Lost Volume (m³)	Max. Outflow (L/s)	Total Discharge Volume (m³)	Percentage Available (%)	Status
Porous Paving	FSR: 100 years: +45 %: 120 mins: Winter	57.423	57.420	0.283	0.300	9.2	17.498	0.000	0.000	2.0	23.350	16.674	OK

Project:	Date: 20/08/2026		
	Designed by: Karol.Gyba	Checked by:	Approved By:
Report Details: Type: Phase Management Storm Phase: Phase	Company Address:		





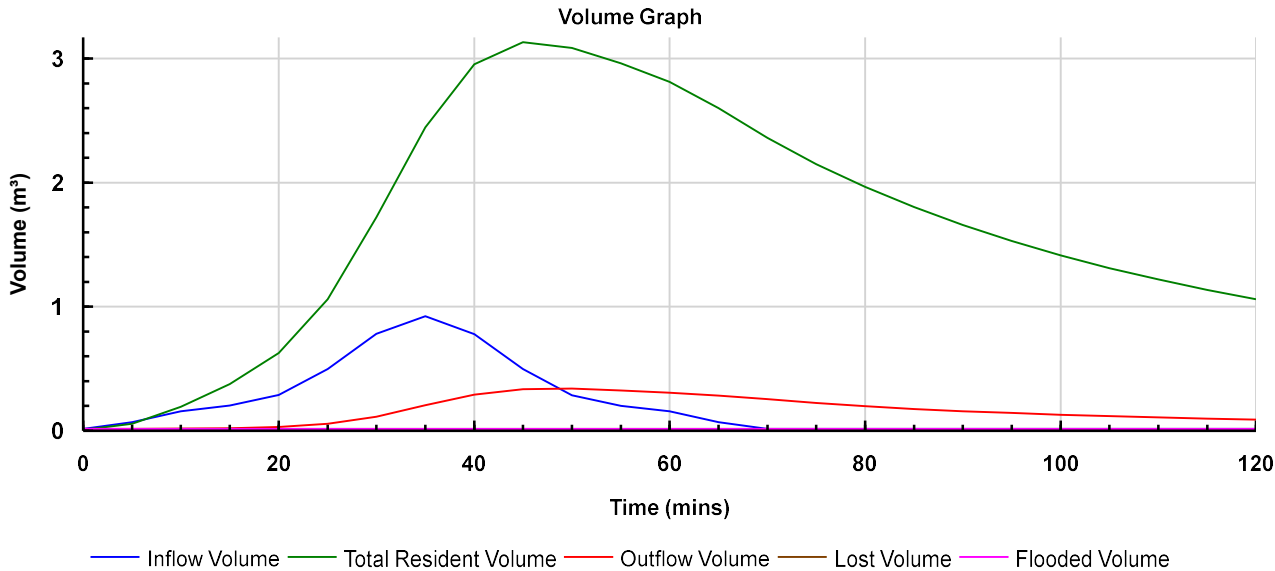
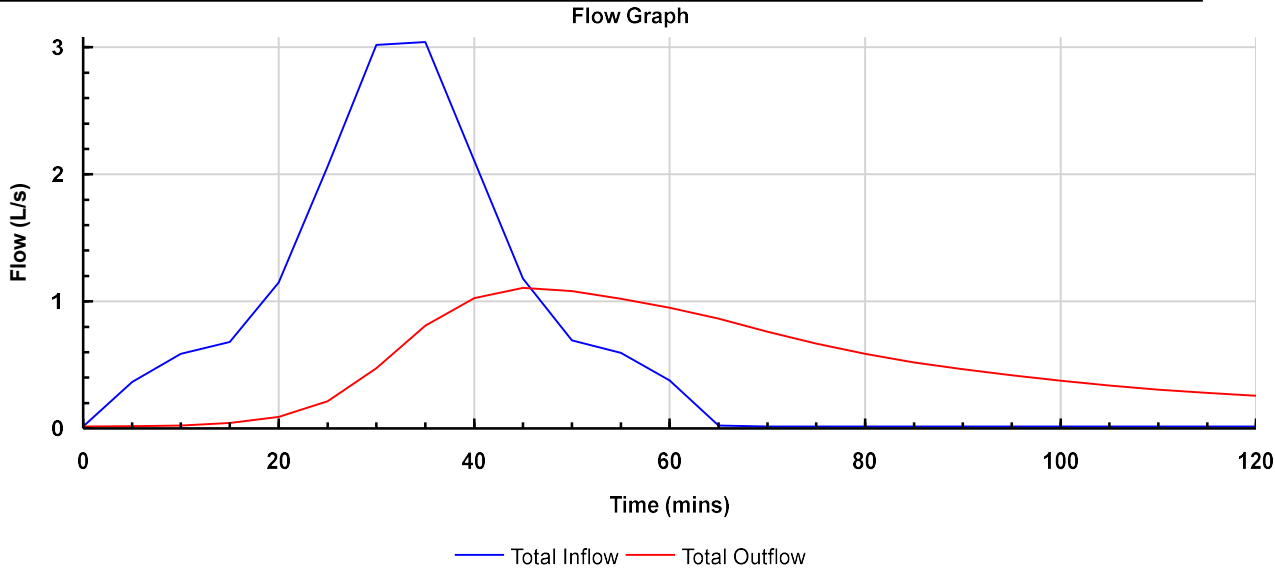
Phase

FSR: 1 years: Increase Rainfall (%): +0: 60 mins: Winter

Tables

Name	Max. Inflow (L/s)	Total Inflow Volume (m³)	Max. Outflow (L/s)	Total Outflow Volume (m³)
Porous Paving			1.1	3.679
TOTAL	3.0	4.728	1.1	3.679

Graphs



Project:	Date: 20/08/2026		
	Designed by: Karol.Gyba	Checked by:	Approved By:
Report Details:	Company Address:		
Type: Phase Management			
Storm Phase: Phase			





Phase

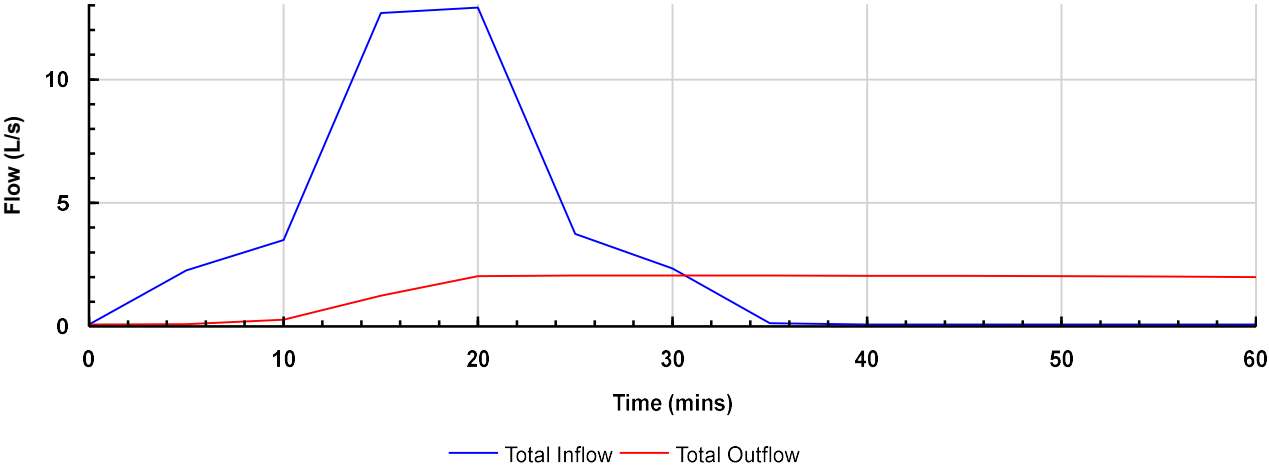
FSR: 30 years: Increase Rainfall (%): +40: 30 mins: Summer

Tables

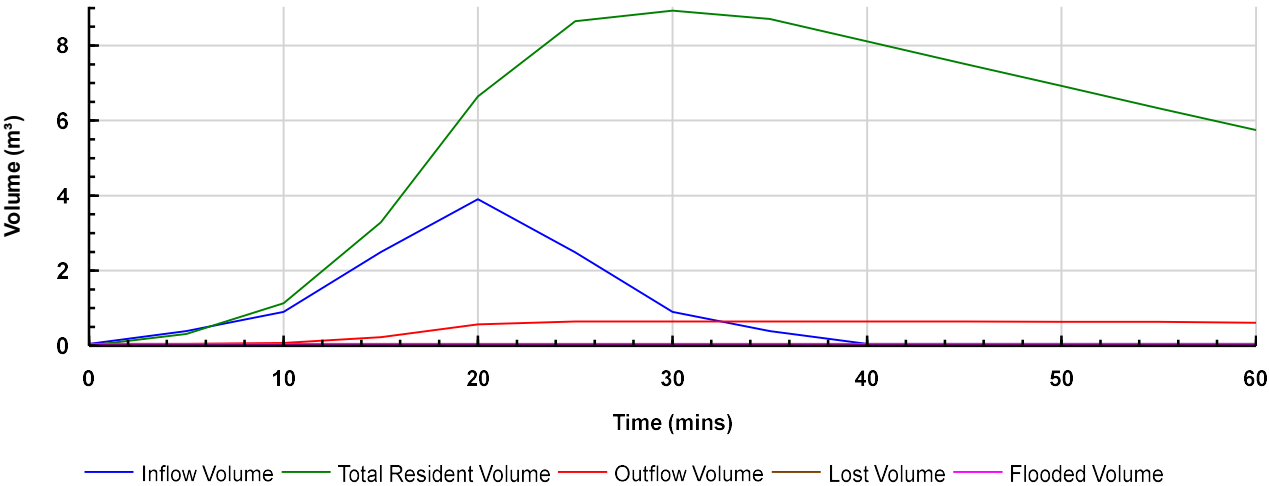
Name	Max. Inflow (L/s)	Total Inflow Volume (m³)	Max. Outflow (L/s)	Total Outflow Volume (m³)
Porous Paving			2.0	5.461
TOTAL	12.9	11.185	2.0	5.461

Graphs

Flow Graph



Volume Graph



Project:	Date: 20/08/2026			
	Designed by: Karol.Gyba	Checked by:	Approved By:	
Report Details:	Company Address:			
Type: Phase Management Storm Phase: Phase				



Phase
FSR: 100 years: Increase Rainfall (%): +45: 15 mins: Summer

Tables

Name	Max. Inflow (L/s)	Total Inflow Volume (m³)	Max. Outflow (L/s)	Total Outflow Volume (m³)
Porous Paving			2.0	2.612
TOTAL	25.5	11.274	2.0	2.612

Graphs

